

***Advanced laboratory tests  
on granular materials  
for transportation facilities in cold regions***

**The first China – Japan Mini Workshop  
on High Speed Railway Geotechnics  
14 December 2015, Beijing, CHINA**

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**International Society for Soil Mechanics and  
Geotechnical Engineering**



# TC202 Executive Group Members (2013 – )



**Chair:**  
**Prof. Erol Tutumluer**  
**University of Illinois at**  
**Urbana-Champaign**



**ex-Chair:**  
**Prof. António**  
**Gomes Correia**  
**University of Minho**



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**University of Wollongong**

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**The University of Texas at El Paso**

**Prof. William Powrie**  
**The University of Southampton**

# TC202 Terms of Reference

The goal of TC202 is to apply broad engineering to bridge the gap between Pavement/Railway Engineering and Geotechnical Engineering. The main task is to promote co-operation and exchange of information and knowledge about the geotechnical aspects in design, construction, maintenance, monitoring and upgrading of roads, railways and airfields. The task also covers the related environmental aspects. For these purposes, several main topics were identified and materialized in the main Task Forces of the Committee as follows.

**TF 1.** Promote of use of nontraditional materials in embankments and structural layers.

**TF 2.** Reinforcement of geomaterials and its implications in pavement and rail track design.

**TF 3.** Earthworks design, technology and management

**TF 4.** Rail track substructures, including transition zones.

**TF 5.** Subsurface sensing for transportation infrastructure condition diagnostics among others.

**TF 6.** 3rd International Conference on Transportation Geotechnics (ICTG), Guimarães, Portugal, September 4-7, 2016. Chairman António Gomes Correia (University of Minho).

**TF 7.** Transportation Geotechnics; Elsevier journal of TC202. Editors-in-Chief: António Gomes Correia; Erol Tutumluer, and Yunmin Chen.

<http://www.journals.elsevier.com/transportation-geotechnics/>

# TF 6. 3<sup>rd</sup> ICTG



**3rd ICTG**  
International Conference  
On Transportation  
Geotechnics

Portuguese Geotechnical Society  
University of Minho



Guimarães, PORTUGAL  
4-7 September 2016  
Vila Flor Cultural Centre

ISSMGE  
International Society for Soil  
Mechanics and Geotechnical  
Engineering



## 3<sup>rd</sup> International Conference on Transportation Geotechnics

### CALL FOR SPONSORS AND EXHIBITORS REGISTRATION

*Deadline for paper submission (accepted abstracts), November 6, 2015*



#### Contact

3rd ICTG Secretariat  
University of Minho - School of Engineering  
Campus de Azurém, 4800-058 Guimarães - PORTUGAL  
Tel.: (+351) 253 510 218  
Fax: (+351) 253 510 217  
e-mail: 3ictrgeo2016@civil.uminho.pt

*Exhibit space is available to  
showcase your activities,  
products and services to  
Academics, Researchers and  
Professionals (more than 300  
abstracts from representatives  
of 50 countries).*

**Conference Website: <http://civil.uminho.pt/3rd-ICTG2016>**

#### Important Dates

**March, 2015:** Call for abstracts  
**May 15, 2015:** Deadline abstract submission  
**June 12, 2015:** Notify authors of accepted abstracts  
**Submission of papers:** After abstract acceptance  
**November 6, 2015:** Deadline paper submission  
**February 26, 2016:** Notify authors of accepted papers  
**March 25, 2016:** Submission of final camera-ready papers

#### Conference Themes / Chairpersons

updated on 2015/09/08

- Theme 01:** Optimized geomaterial (including hydraulically bound materials and asphalt mixtures) use, reuse and recycling - *T. Edil, N. Consoli, A. Dawson*
- Theme 02:** Unsaturated soil mechanics in transportation geotechnics - *D. Toll, E. Alonso*
- Theme 03:** Foundations and earth structures - *A. Gomes Correia, H. Brandl*
- Theme 04:** Slope stability, stabilisation, and asset management - *S. Glendinning*
- Theme 05:** Mechanistic-empirical design (road, railways and airfields) - *C. Schwartz, D. Brill, S. Costa d'Aguiar*
- Theme 06:** Rail track substructures, including transition zones - *W. Powrie, M. Shahim*
- Theme 07:** Subsurface sensing for transportation infrastructure - *S. Nazarian, A. Loizos*
- Theme 08:** Macro and Nanotechnology applied to transportation geotechnics - *M. Alves, J.M. Fleureau*
- Theme 09:** Sustainability in transportation geotechnics - *I. Al-Qadi, M. Winter*
- Theme 10:** Case histories - *J. Koseki, J. Oliveira, J. Liu*

#### Workshops Themes

- Workshop 01:** Geosynthetics in transportation geotechnics
- Workshop 02:** Harbour geotechnics
- Workshop 03:** Non destructive technologies
- Workshop 04:** Ground improvement and soil stabilization

#### Honour Lectures

ISSMGE - **TC202: Proctor Lecture:** Prof. Buddhima Indra Pratna (Australia)  
IGS: **Mercer Lecture:** Prof. Jorge Zornberg (USA)

#### Co-Sponsors

updated on 2015/09/08



# TF 7. Issue of International Journal

# NEW JOURNAL TRANSPORTATION GEOTECHNICS

ISSMGE TC 202

EDITORS



**Professor Antonio  
Gomes-Correia,**  
University of Minho,  
Guimarães, Portugal

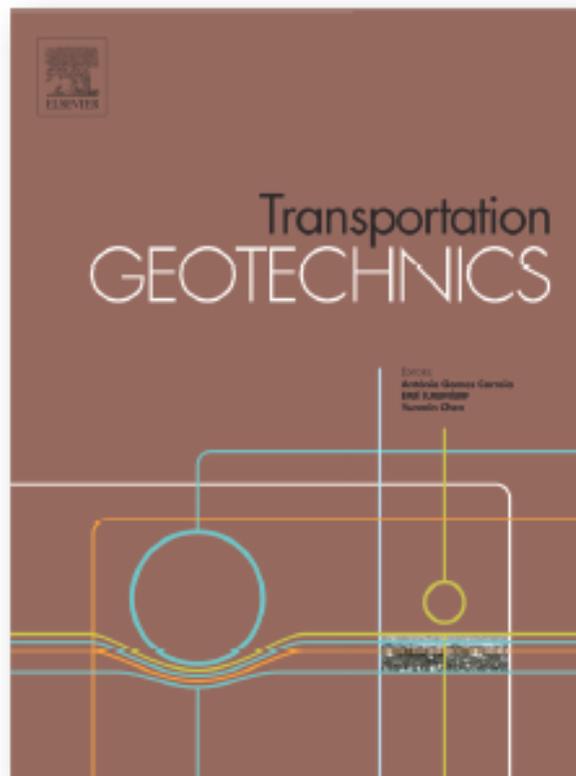
1<sup>st</sup> issue  
03/2014

*Transportation Geotechnics* is a new journal publishing high quality theoretical and applied papers on all aspects of geotechnics for roads, highways, railways, airfields and waterways.

**Editors:**

António Gomes Correia  
Erol Tutumluer  
Yunmin Chen

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[www.elsevier.com/locate/trgeo](http://www.elsevier.com/locate/trgeo)



**Professor Erol  
Tutumluer,**  
University of Illinois,  
Urbana-Champaign,  
USA



**Professor Yunmin  
Chen,**  
Zhejiang University,  
Zhejiang, China

# Technical Committee 202

## TC202 Members:

51 members covering a good majority of ISSMGE member societies

In Asia:	China	2 members	Iran	1 member
	Chinese Taipei	1	Japan	4
	Hong Kong	2	Kazakhstan	1
	India	1	South Korea	3
	Indonesia	1	<b>Total</b>	<b>17 members</b>

## TC202 Web Sites:

<http://www.issmge.org/en/technical-committees/applications/147-transportation-geotechnics>

<http://www.eng.hokudai.ac.jp/labo/geomech/ISSMGE%20TC202/>

## Correspondence:

Dr. Tatsuya Ishikawa  
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International Society for Soil Mechanics and Geotechnical Engineering  
ISSMGE  
TC202  
Transportation  
Geotechnics

1 Jun 2014 Update applied to reflect the new TC regime launched last year.

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**WELCOME TO TC202**  
Host member society: USA  
Short name: Transportation (TC202)

Technical Committee 202 (formerly TC-3) of the ISSMGE was established in 2001 in accordance with the proposal approved by the ISSMGE Board serving the 2001-2005 term. The Committee completed three 4-year terms under the leadership of Professor Antonio Gomes Correia as Chairman (2001-2013). Professor Erol Tutumluer is the current Chairman of TC202 serving the 2013-2017 term.

Technical Committee 202 works closely with its members to organize symposia, workshops, and international exchanges dealing with all aspects of Transportation Geotechnics in both traditional and emerging areas. Thanks to the dedicated efforts of Prof. Gomes Correia, TC202 has created a new named lecture series in commemoration to late R.R. Proctor to be delivered by the world's most distinguished achievements in Transportation Geotechnics. The inaugural Proctor Lecture will be delivered at the 3rd International Conference of Transportation Geotechnics in Guimaraes, Portugal, on September 4-7, 2016.

You are encouraged to contact us, bring new proposals, and/or participate in the ongoing activities of TC202.

Prof. Erol Tutumluer, Chair  
Prof. Tatsuya Ishikawa, Secretary

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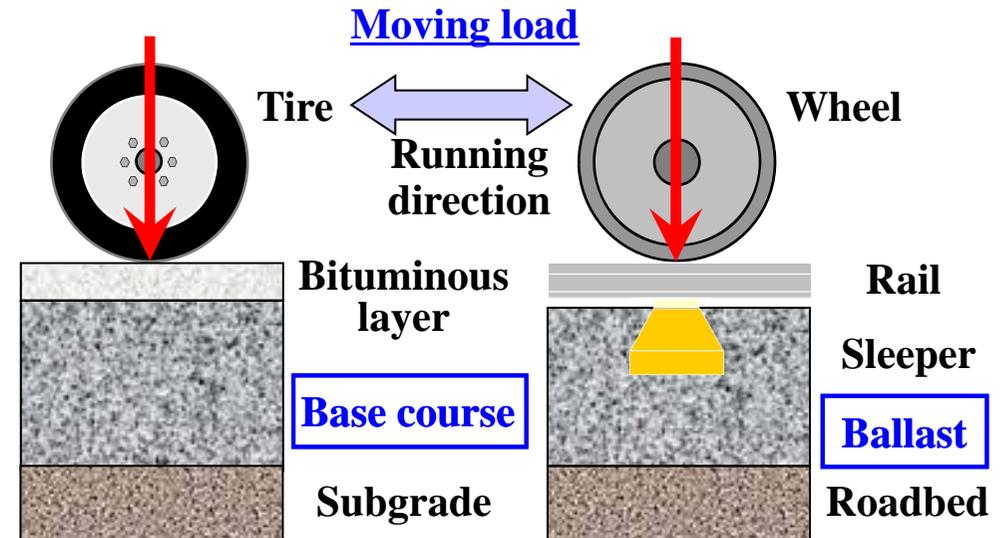
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# Background of research (1)

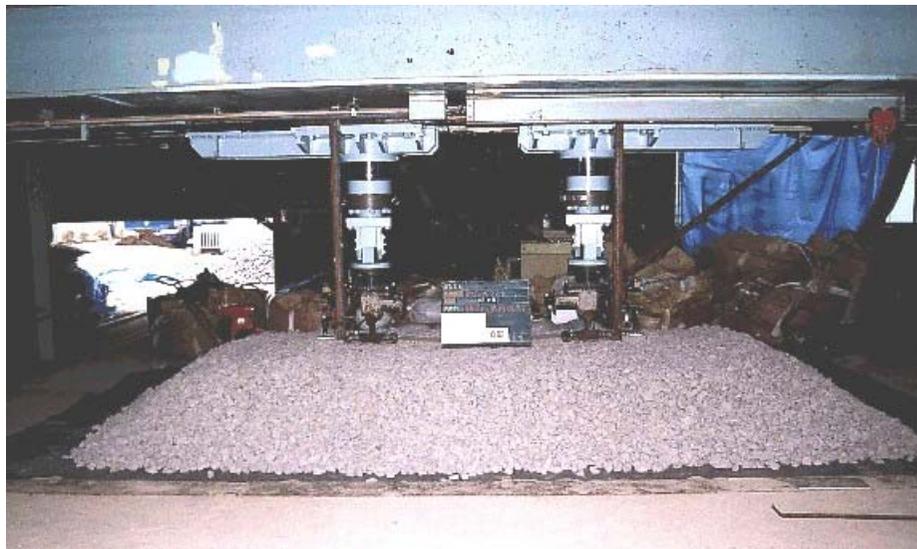
For saving maintenance costs of transportation facilities at road and railway, it is important to elucidate mechanical behavior of granular roadbed subjected to traffic loads.

## Conventional studies

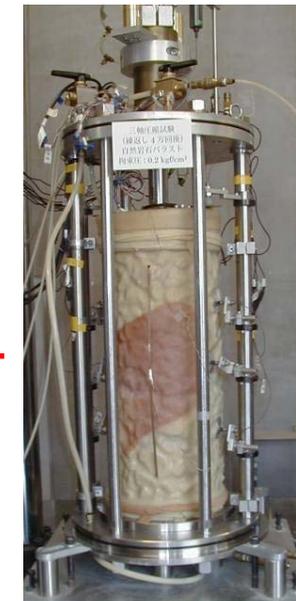
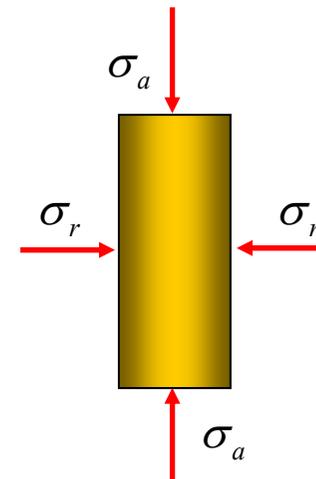


Following loading tests have been performed in Japan;

Model test : [fixed-point loading test](#)



Element test : [Triaxial test](#)



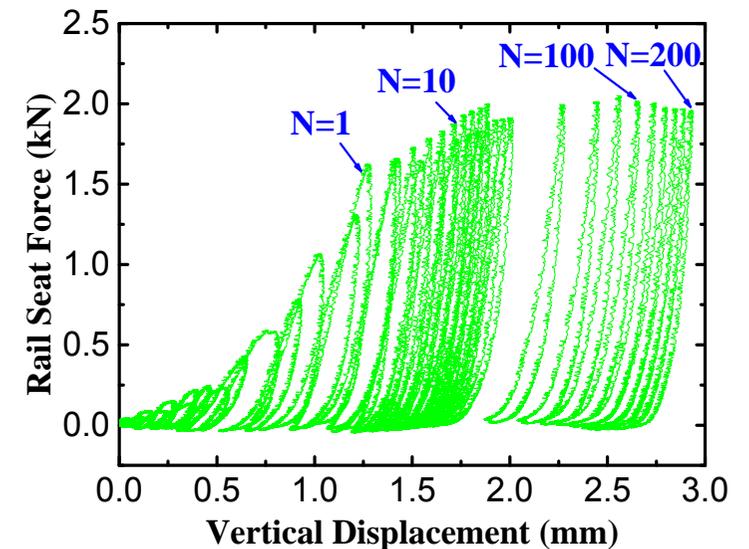
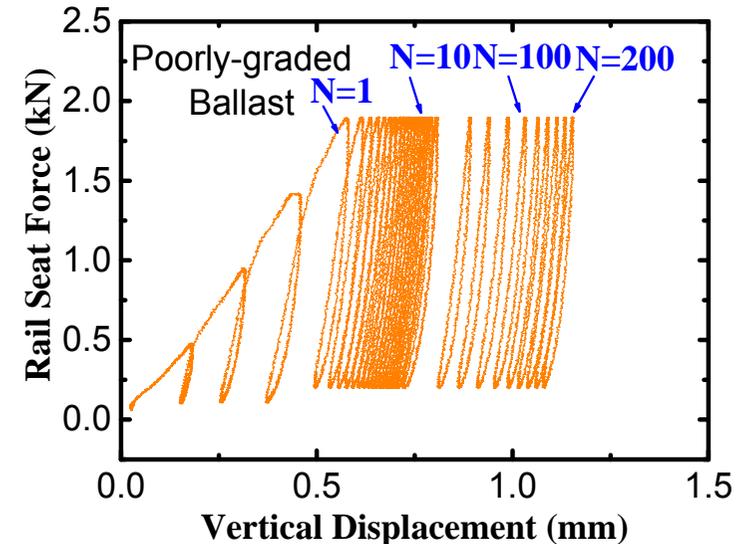
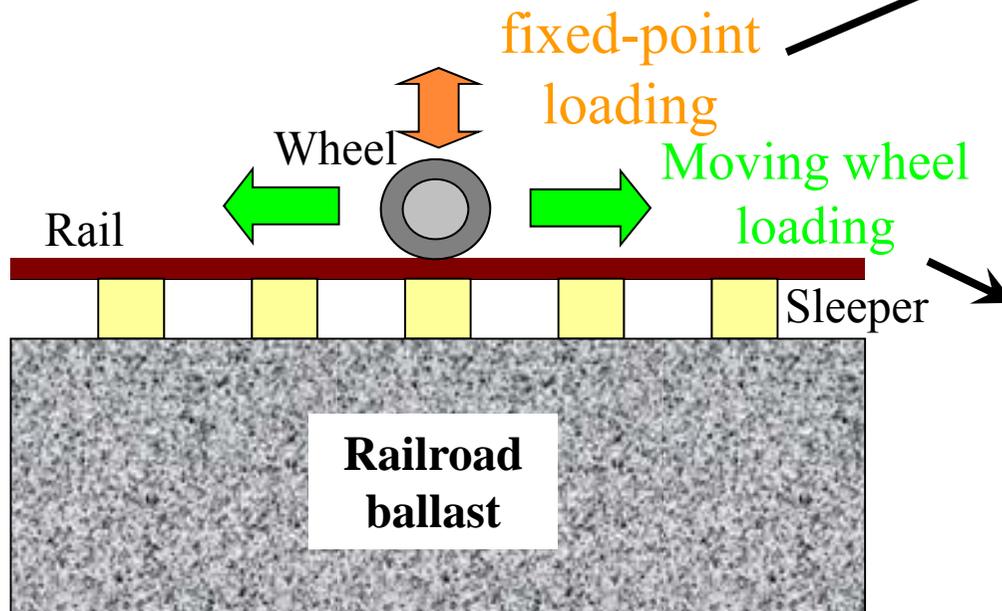
# Effect of moving wheel loads

## KEY POINTS

- Can fixed-point loading tests simulate actual traffic loads ?

## Literature Review

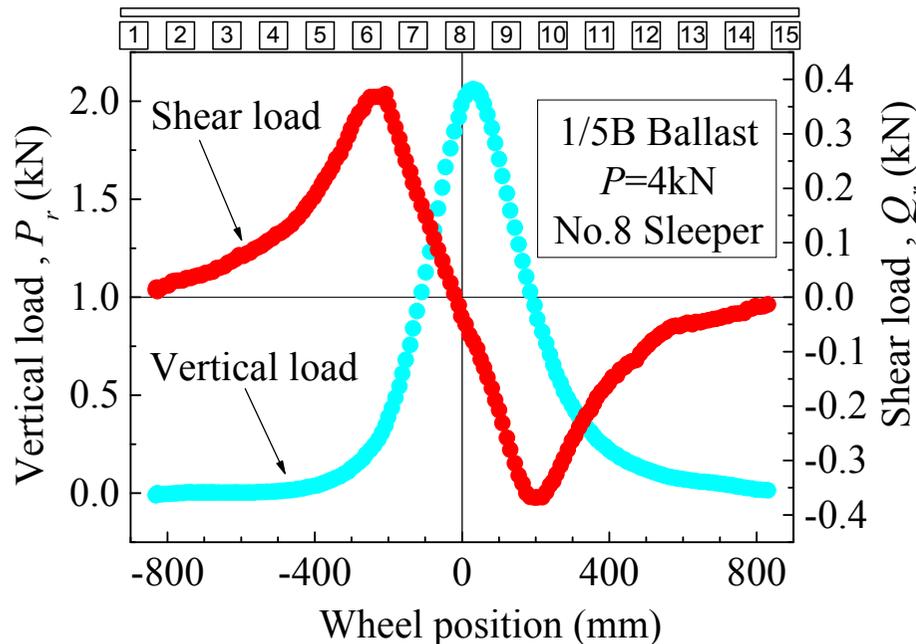
Ishikawa & Sekine 2007



Moving wheel loads give a large residual settlement as compared with fixed-point cyclic loads.

# Moving wheel load

## Moving wheel loading test

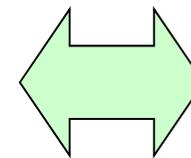


## Measured Ballast Pressure

- Normal component remains compression side through loading, while shear component changes the sign according to the position of a loading wheel.
- Principal stress axes rotate inside railroad ballast during train passage.

fixed-point loading

Increase & decrease of loads



Moving wheel loading

+ Principal stress axis rotation

# Change in water content

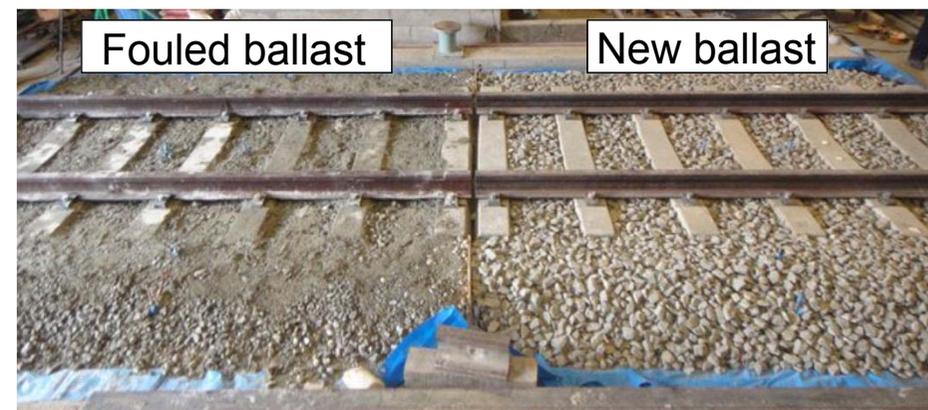
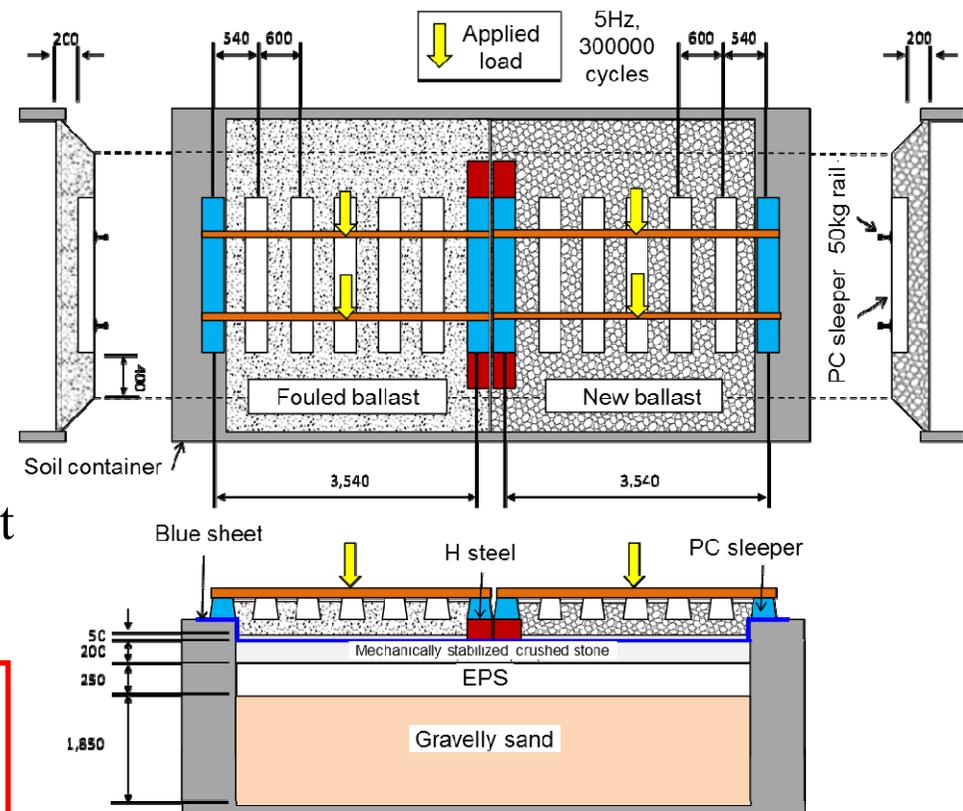
## Literature Review

- In past studies, model and element tests have been performed mainly under air-dried condition.
- Deformation modulus of coarse granular materials decreases with the increase in water content. (Coronado et al. 2005, Ekblad & Isacsson 2006)

## KEY POINTS

- Does rainfall affect settlement of railroad ballast ?

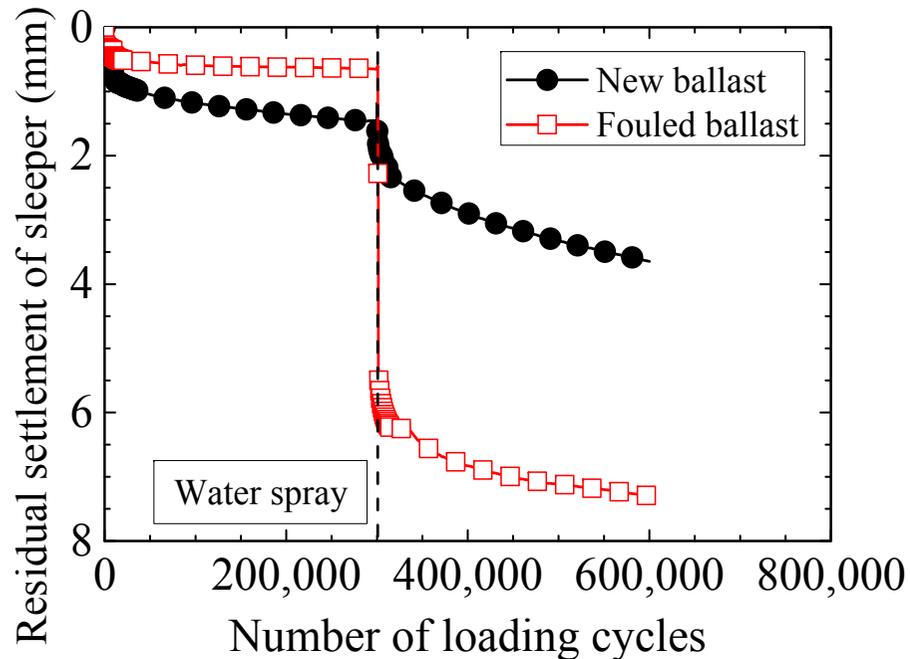
➔ Itou et al. (2014) performed cyclic loading tests of full-scale model ballasted track before and after spraying water on the surface of railroad ballast ( $10 \text{ l/m}^2$ ).



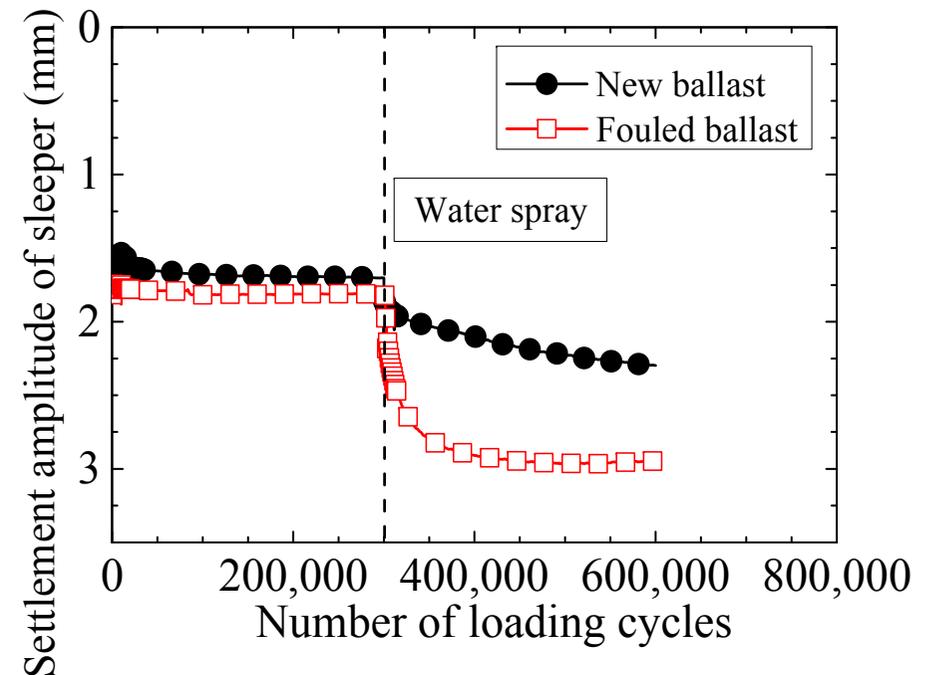
Full-scale model test

# Results of full-scale model tests

## Influence of water content



Residual settlement (Plastic behavior)

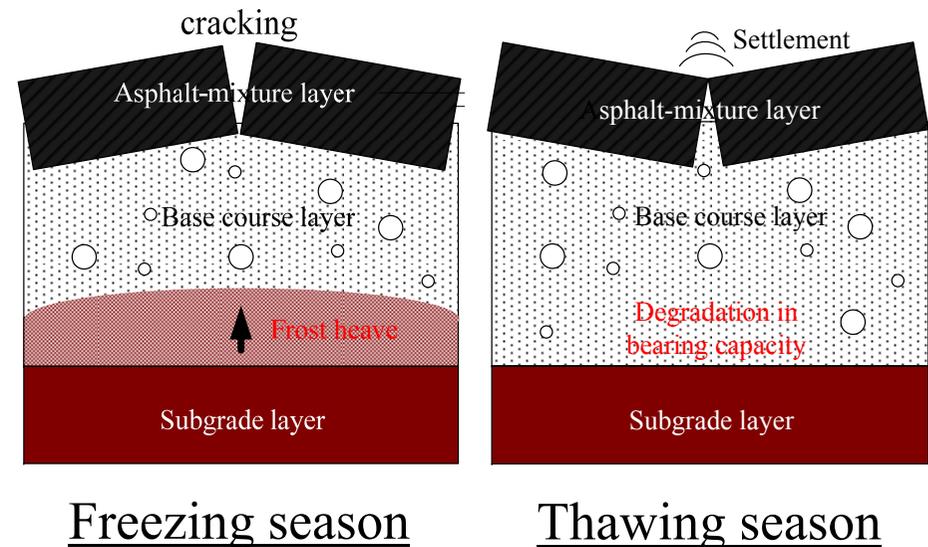
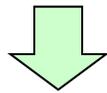


Settlement amplitude (Elastic behavior)

- Plastic deformation and elastic deformation of ballasted track during cyclic loading suddenly increase due to water spray.
- Wetting causes the sample to decrease the shear stiffness, thereby increasing residual settlement.

## Background of Research (2)

In snowy cold regions, frost-heave phenomenon and temporary degradation in the bearing capacity during thawing season are thought to accelerate the deterioration of transportation facilities and losing of the functions.



However, in Japan, such phenomena have not been sufficiently elucidated as well as modeling for theoretical design method of transportation facilities like railway track and pavement structures.

### Research topics

To develop a rational design method of asphalt pavement and ballasted track in cold regions, we need to understand the mechanical behavior of subgrade and base course during freeze-thawing in detail.

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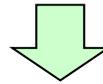
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# Objectives of Research

## **KEY POINTS**

- **Are results obtained from conventional laboratory tests credible ?**
- **How about the synergistic effects of multiple factors ?**



## **Research objective**

**To evaluate effects of water content, principal stress axis rotation, loading rate, and freeze-thaw history on mechanical behavior of granular materials in terms of **Transportation Geomechnics**.**

## **Research content**

- Medium-size triaxial compression tests of coarse granular materials under various degrees of saturation and different loading rates
- Multi-ring shear tests of granular materials under various degrees of saturation subjected to principal stress axis rotation
- CBR tests and Multi-ring shear tests of freeze-thawed coarse granular materials under various degrees of saturation

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# Medium-size triaxial test apparatus

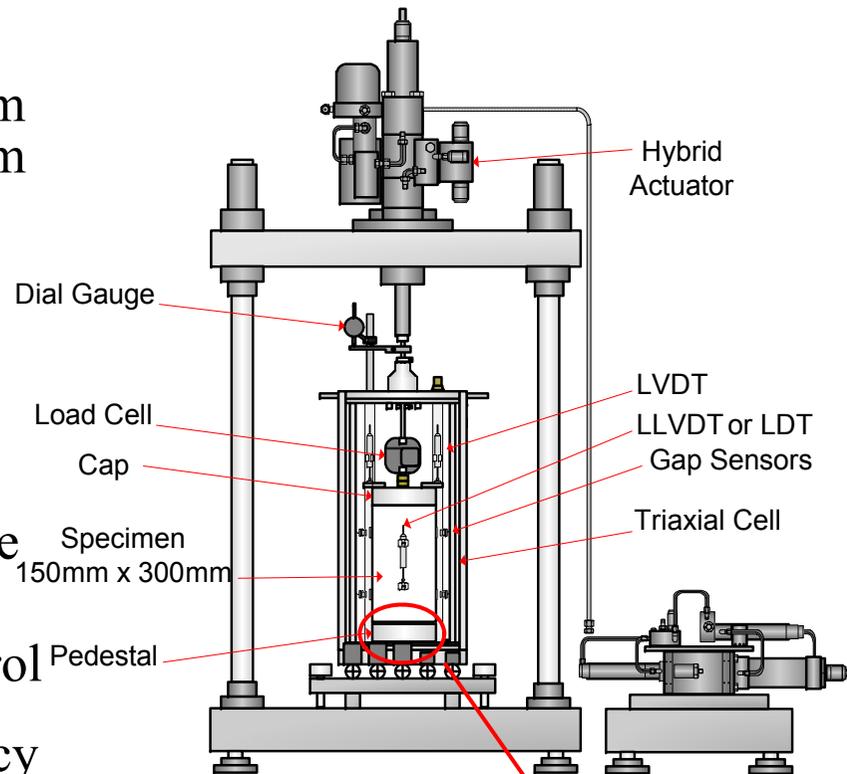
## Specimen size

Diameter: 150mm  
Height: 300mm

## Test condition

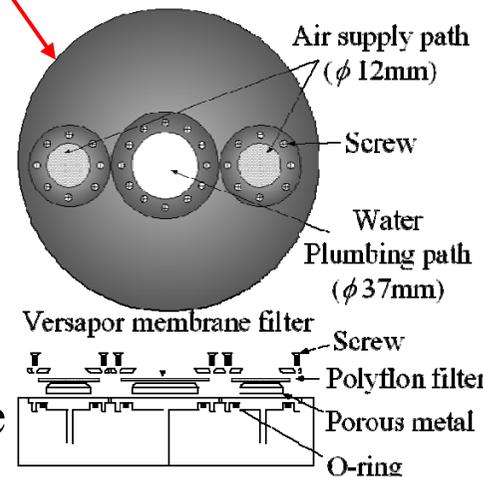
Suction-controlled

- Monotonic loading with strain control method at very slow loading rate
- Cyclic loading with stress control method at high loading frequency



## Remarkable Features

- The apparatus can perform water retentivity tests, triaxial compression tests and resilient modulus test (MR test) for unsaturated soils to evaluate the deformation-strength characteristics of coarse granular materials.
- The apparatus adopts pressure membrane method for reducing total testing time.



# Multi-ring shear test apparatus

- Specimen size

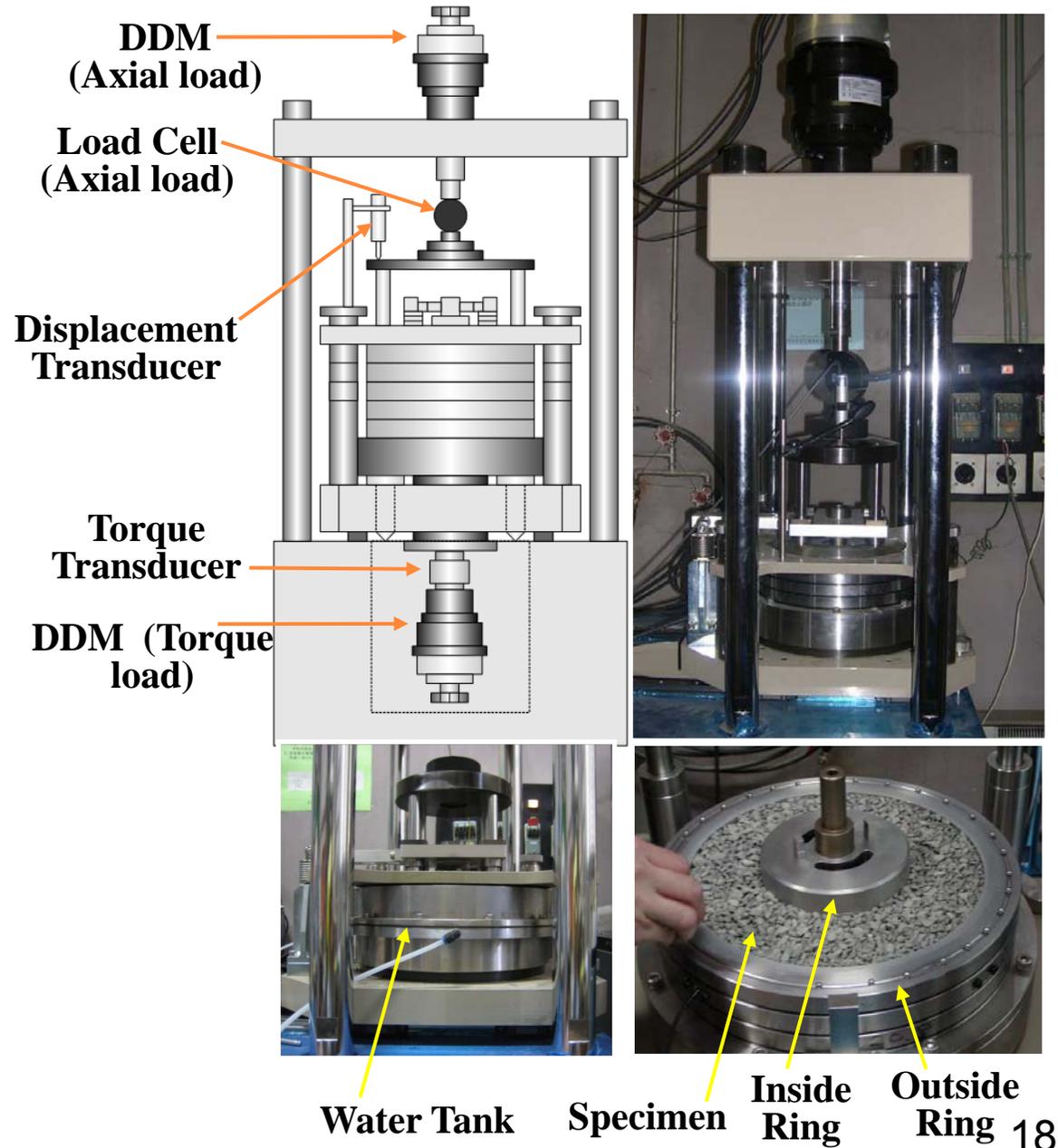
Width: 60mm  
    inside D=120mm  
    outside D=240mm  
Height: 60 - 100mm

- Test condition

Torsional simple shear with strain-control and stress-control method using direct drive motor (DDM)

- Remarkable Features

- It can evaluate the effects of rotating principal stress axes under shearing on the deformation-strength characteristics of granular materials.
- It can perform monotonic and cyclic loading tests under various water contents and freeze-thaw history.



# Freeze-thaw CBR test apparatus

- Specimen size

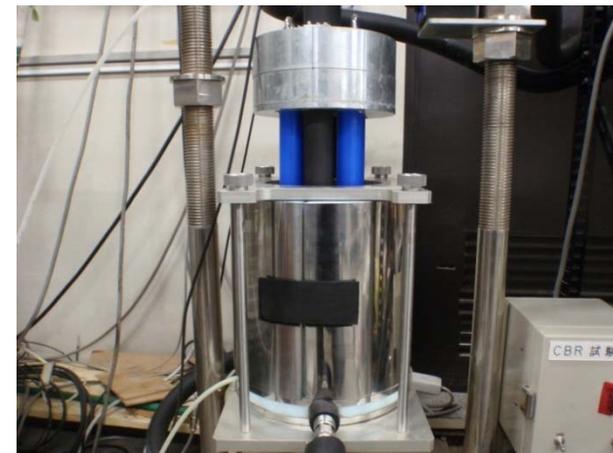
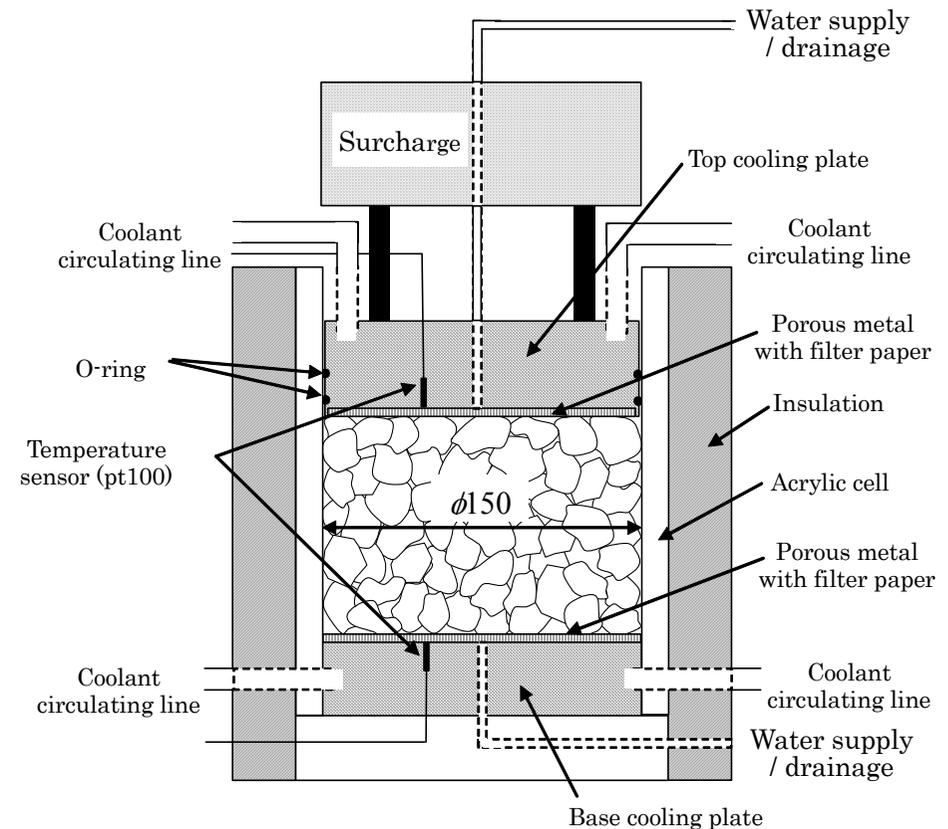
Width: 60mm  
    inside D=120mm  
    outside D=240mm  
Height: 60 - 100mm

- Test condition

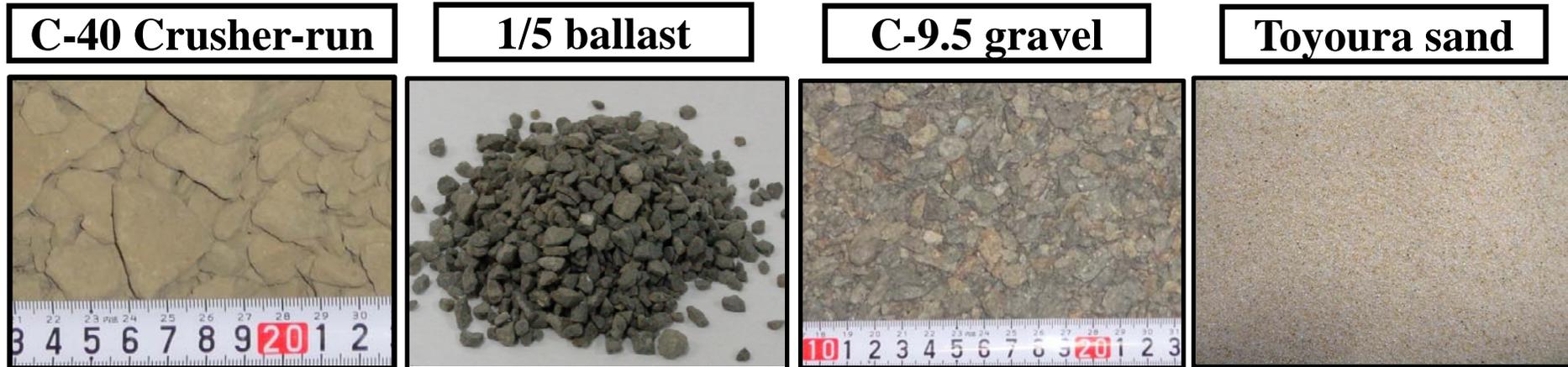
It can conduct a CBR test as it is immediately after the freeze-thaw.

- Remarkable Features

- It can subject a CBR test specimen ( $\phi=15\text{cm}$ ,  $H=12.5\text{cm}$ ) to a one-dimensional desired freeze-thaw history at a constant freezing rate.
- It allows open-system freezing or closed-system freezing.



# Test Samples



Sample	$\rho_s$ (g/cm <sup>3</sup> )	$\rho_{dmax}$ (g/cm <sup>3</sup> )	$\rho_{dmin}$ (g/cm <sup>3</sup> )	D <sub>50</sub>	U <sub>c</sub>	F <sub>cinitial</sub> (%)
C-40 crusher-run	2.74			9.10	37.1	1.7
1/5 ballast	2.70	1.650	1.353	8.11	1.52	0.0
C-9.5 gravel	2.72	1.730	1.480	2.80	10.8	0.0
Toyouura sand	2.65	1.648	1.354	0.18	1.30	0.0

- Four types of test samples are employed to examine the mechanical behavior of railroad ballast and roadbed at ballasted tracks.
- To ensure experimental accuracy in terms of specimen size, 1/5 ballast and C-9.5 gravel have one-fifth and one-fourth similar grading of 1/1 ballast and base course material (C-40) used in Japanese railway and road, respectively.

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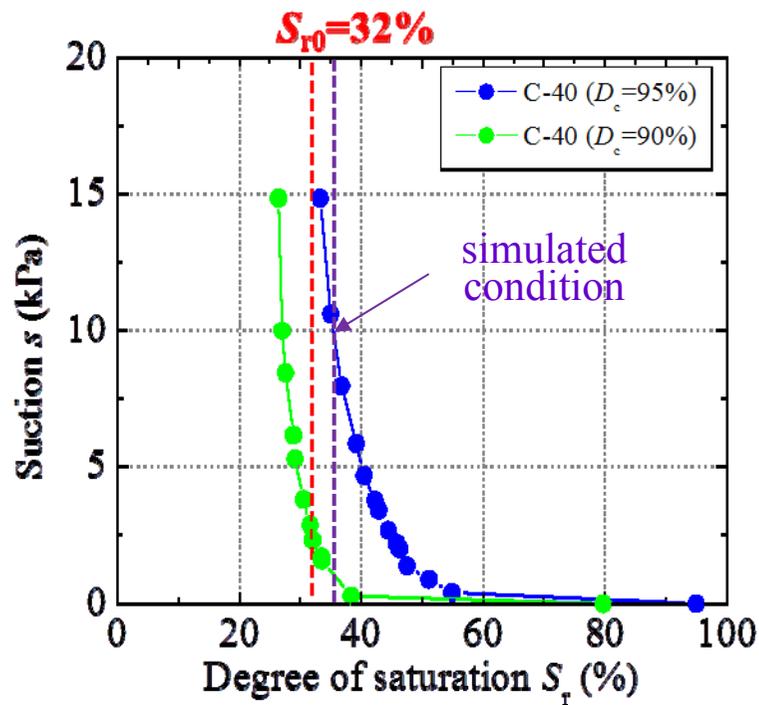
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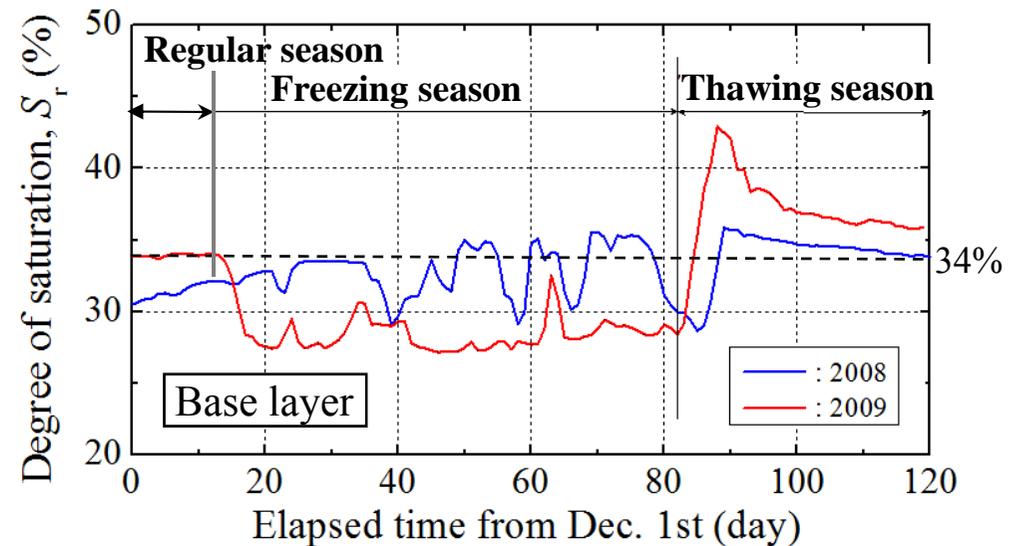


# Testing Method

- Specimens were prepared by compacting with vibrator until reaching the  $D_c$  of 95 %, and they were kept throughout the test at a specified degree of saturation by controlling matric suction.
- Triaxial compression tests for air-dried ( $S_r=8.2\%$ ), simulated ( $S_r=36.7\%$ ), optimum ( $S_r=57.2\%$ ) and saturated ( $S_r=100\%$ ) specimens were conducted under fully drained condition, pursuant to JGS and AASHTO standards.



SWCC

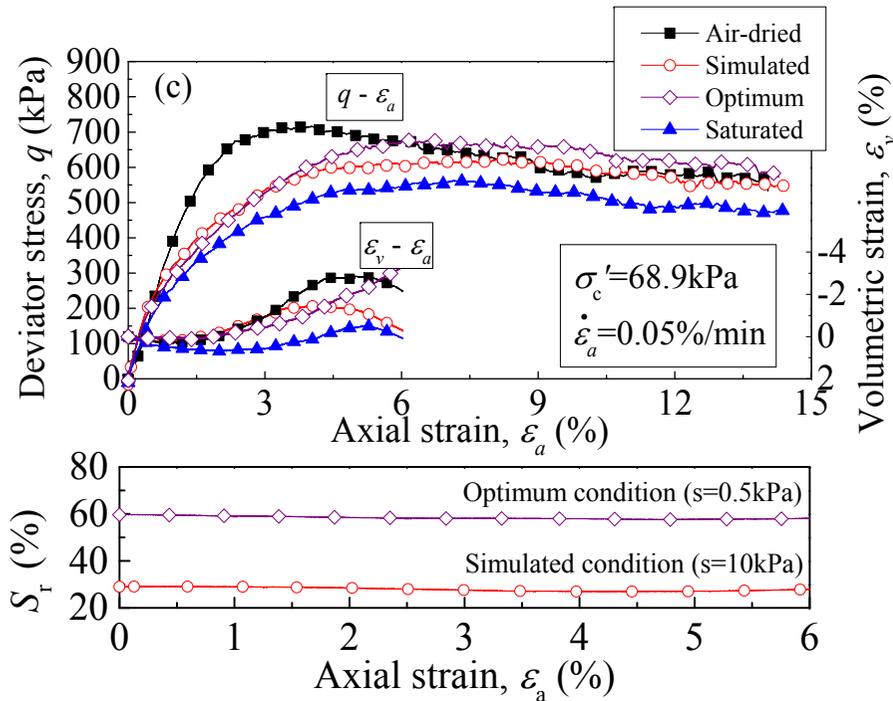


Long-term field measurement

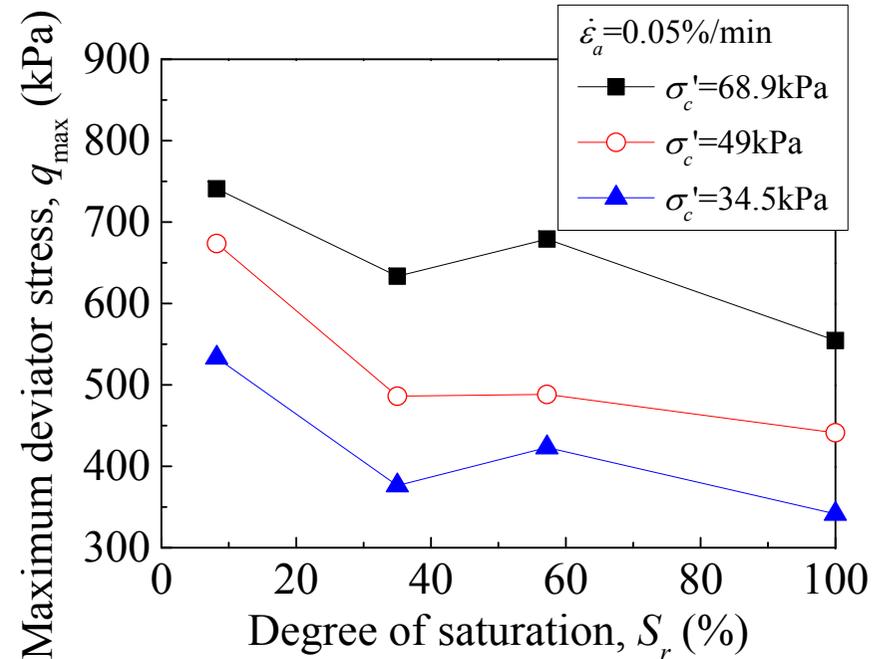
- We set the  $S_r$  for simulated specimens to the value close to  $S_r$  at the in-situ base layer during regular season.

# Results of monotonic (static) loading tests

## Influence of water content



Stress – strain relations under different  $S_r$

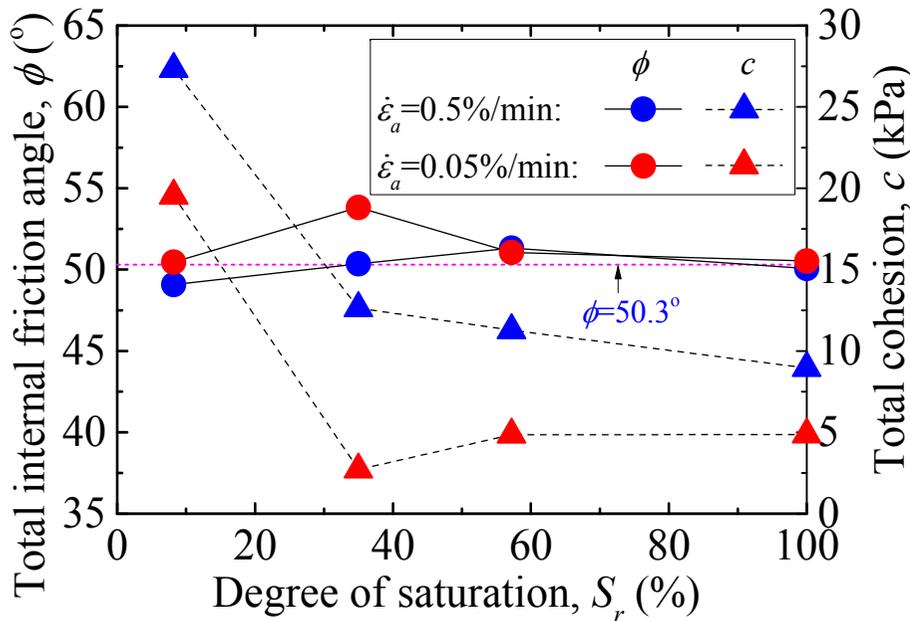


Peak strength at same strain rate under different confining pressures and  $S_r$

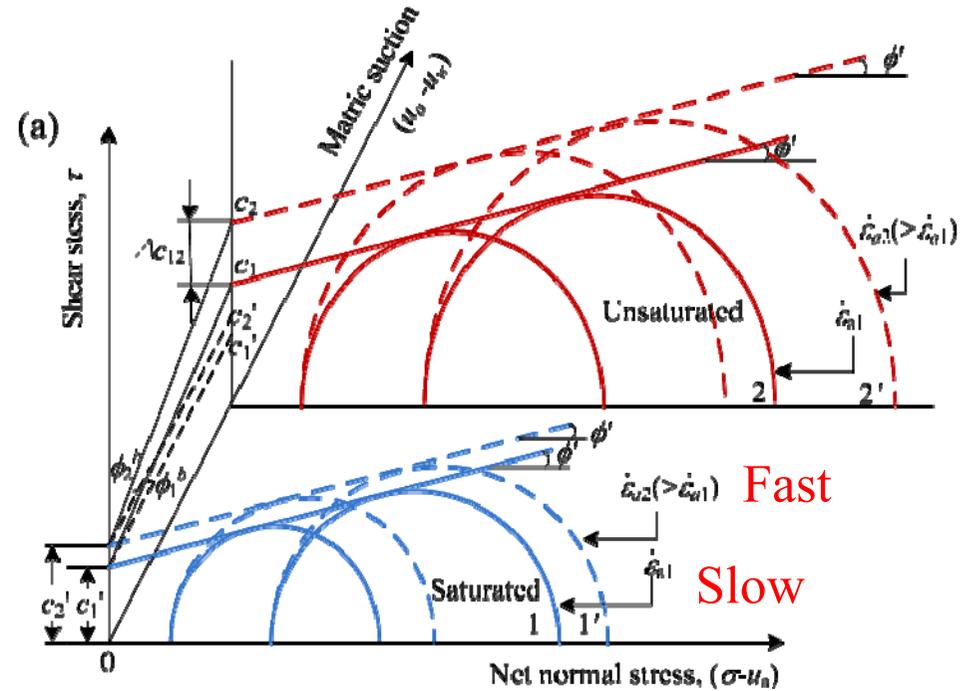
- Under same strain rate, air-dried sample has maximum peak strength, saturated sample has minimum one, and unsaturated samples have a peak strength between those two values, regardless of effective confining pressure.
- Wetting causes the sample to decrease the shear strength and stiffness.

# Results of monotonic (static) loading tests

## Influence of loading rate



Strength parameters under different loading rates and  $S_r$

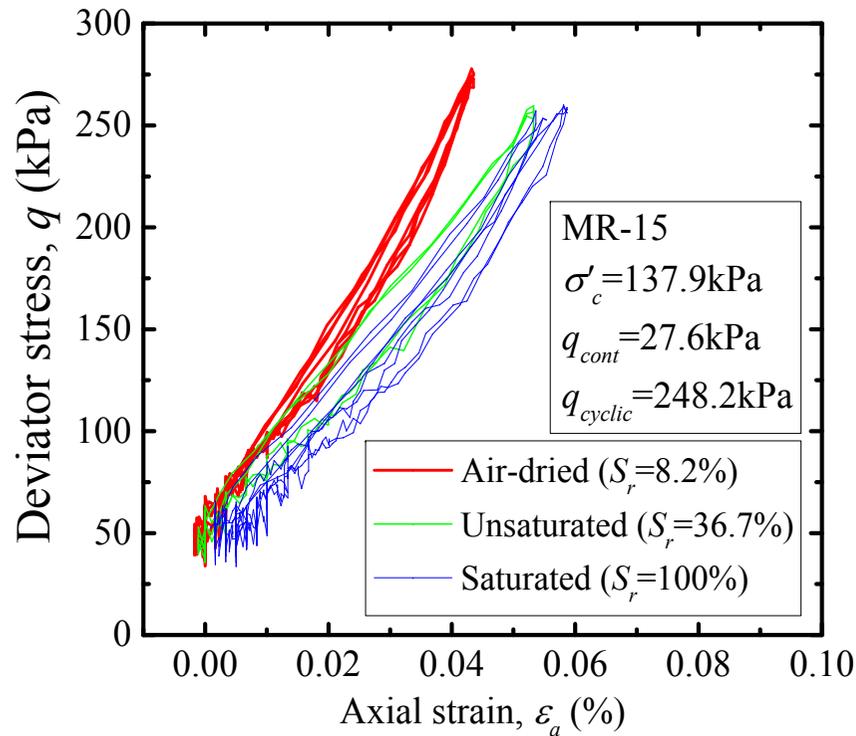


Three-dimensional failure surface for unsaturated soils

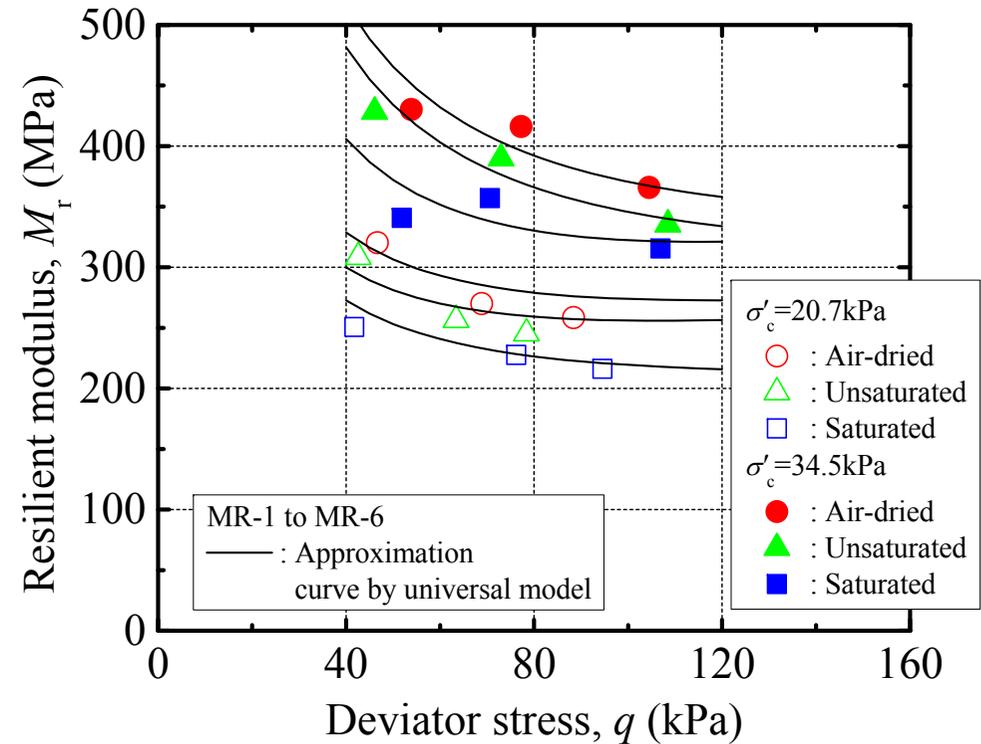
- At same water content, test results under fast loading rate has higher cohesion than test results under slow loading rate, while internal friction angle is almost constant, irrespective of loading rate and degree of saturation.
- The influence of loading rate on the strength parameter for crusher-run appears more clearly in unsaturated conditions.

# Results of resilient modulus tests

## Influence of water content



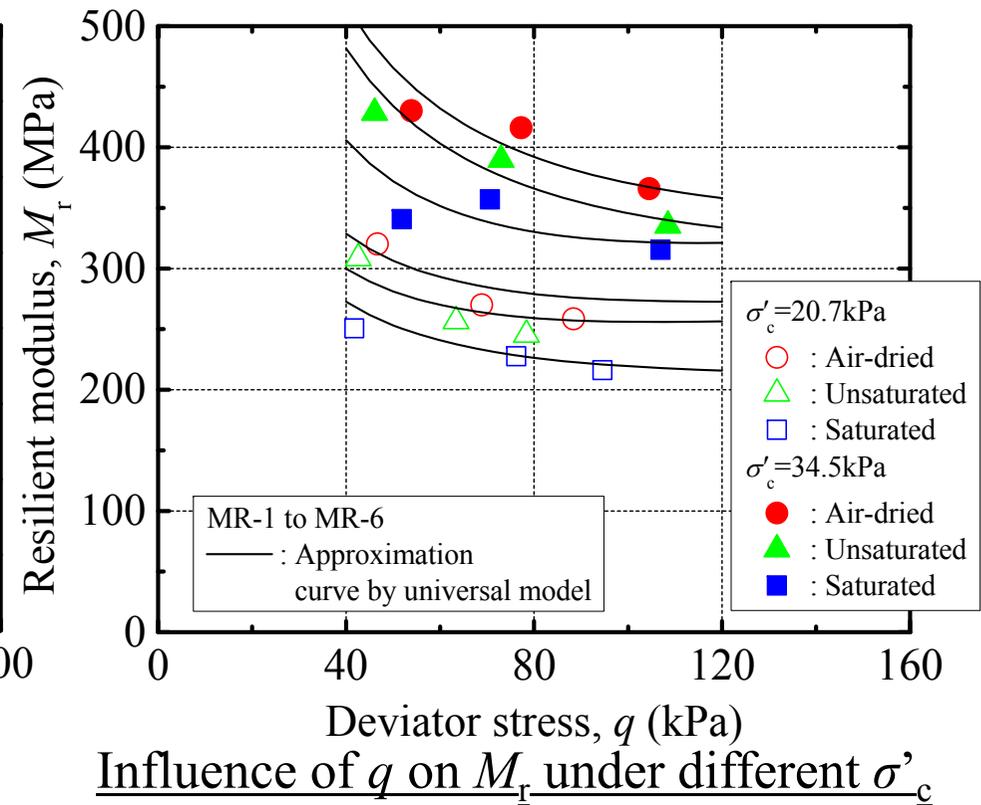
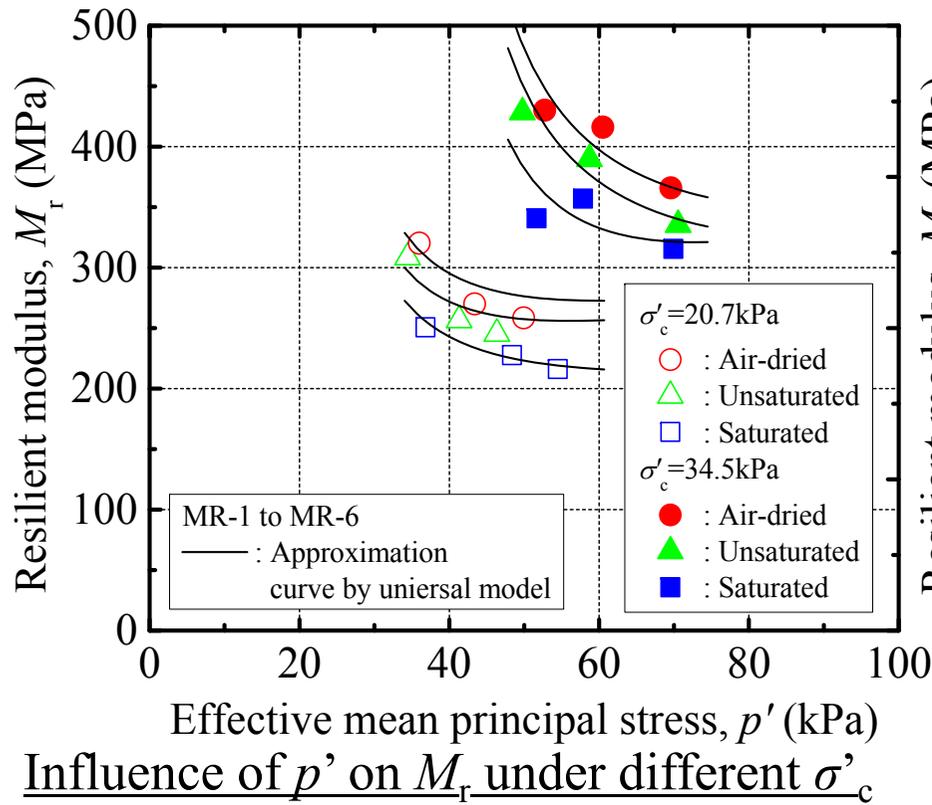
Stress – strain relations under different  $S_r$



Resilient modulus under different  $S_r$

- Stress-strain relationships varies depending on the water content, with a tendency for the stiffness to increase with the decrease in water content.
- For plots with the same  $q$ , resilient modulus decreases as the water content increases, regardless of  $\sigma'_c$ .

# Stiffness estimation of unsaturated C-40



**Universal model:** 
$$M_r = k_1 p_a \left( \frac{\sigma_{ii}}{p_a} \right)^{k_2} \left( \frac{\tau_{oct}}{p_a} + 1 \right)^{k_3} \quad (\text{Yan and Quintus 2002})$$

- Resilient modulus for unbound granular base course materials is greatly dependent on the stress conditions and the water content.
- Universal model given by above equation can be applied to the quantitative evaluation of resilient modulus for unsaturated base course materials.

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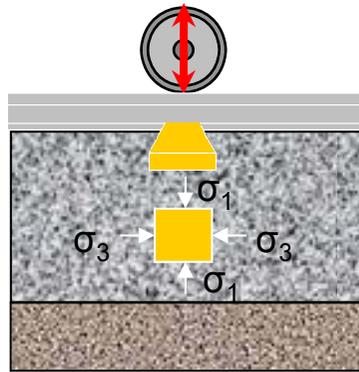
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# Testing Method

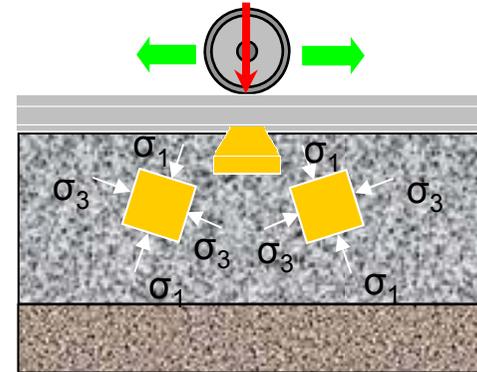
- Specimens were prepared by tamping a sample with prescribed water content, and a series of multi-ring shear tests was conducted by using fixed-point loading (FL) method and moving-wheel loading (ML) method.

## Fixed-point loading (FL)

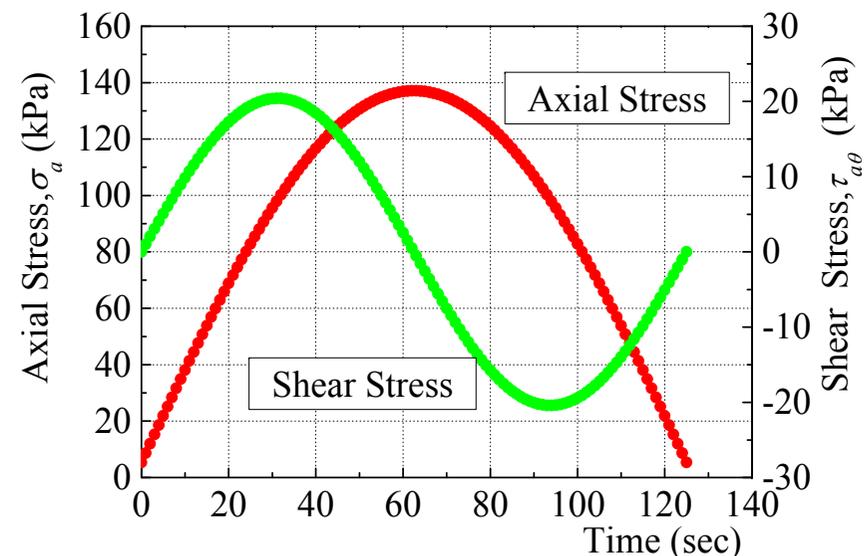
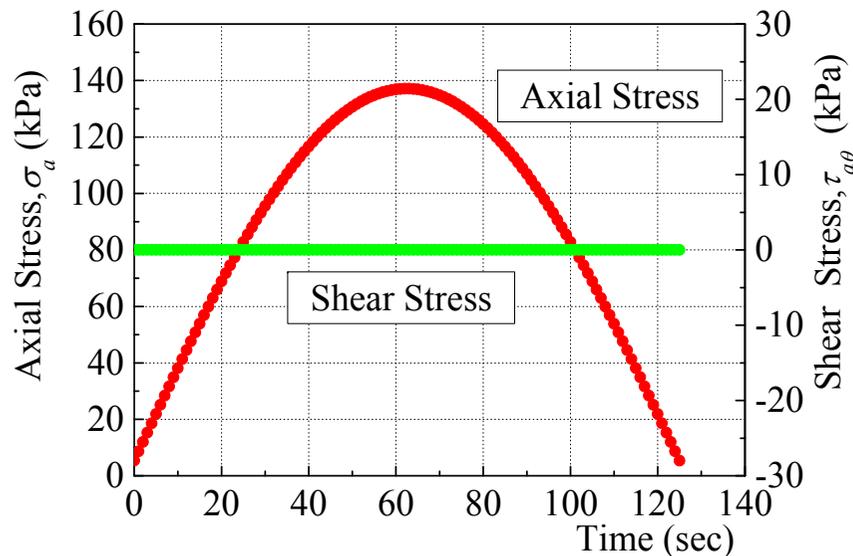


Increase & decrease of loads like triaxial compression test

## Moving-wheel loading (ML)



Increase & decrease of loads with principal stress axis rotation



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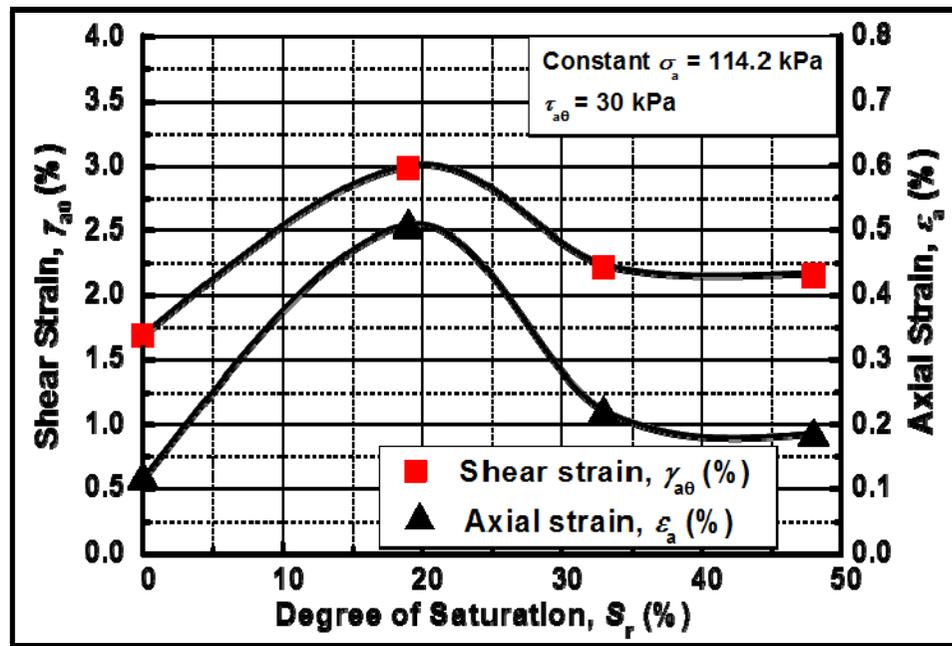
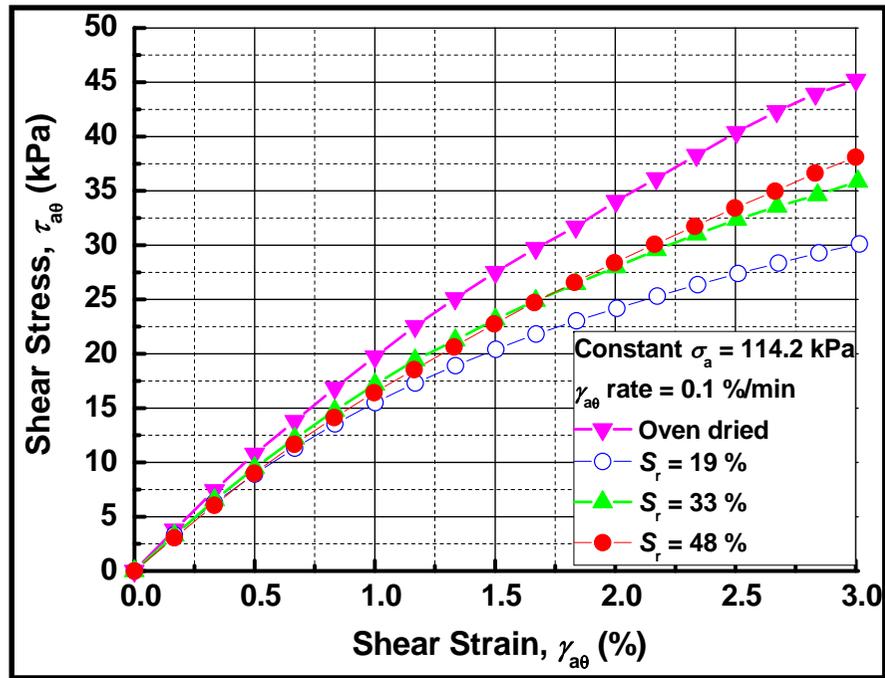
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# Results of monotonic (static) loading tests

## Influence of water content



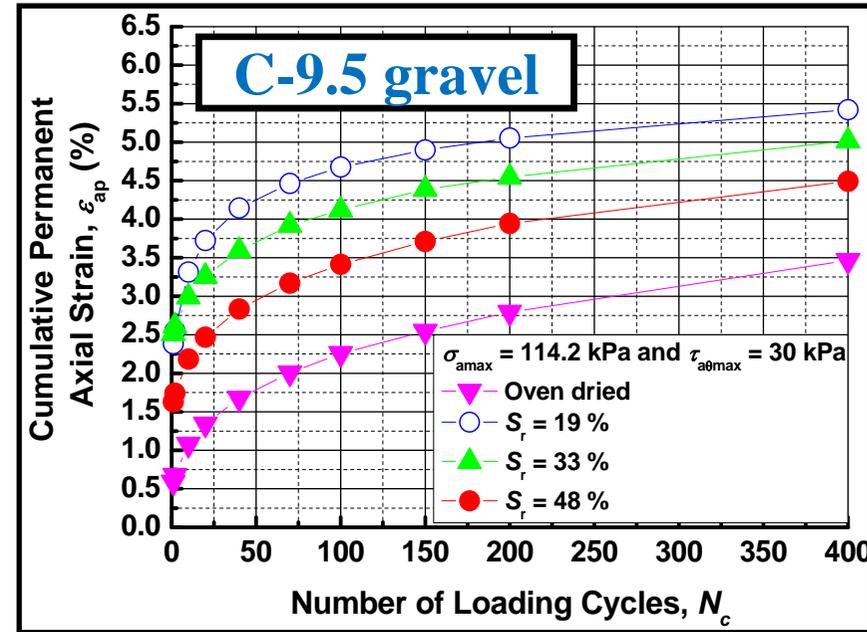
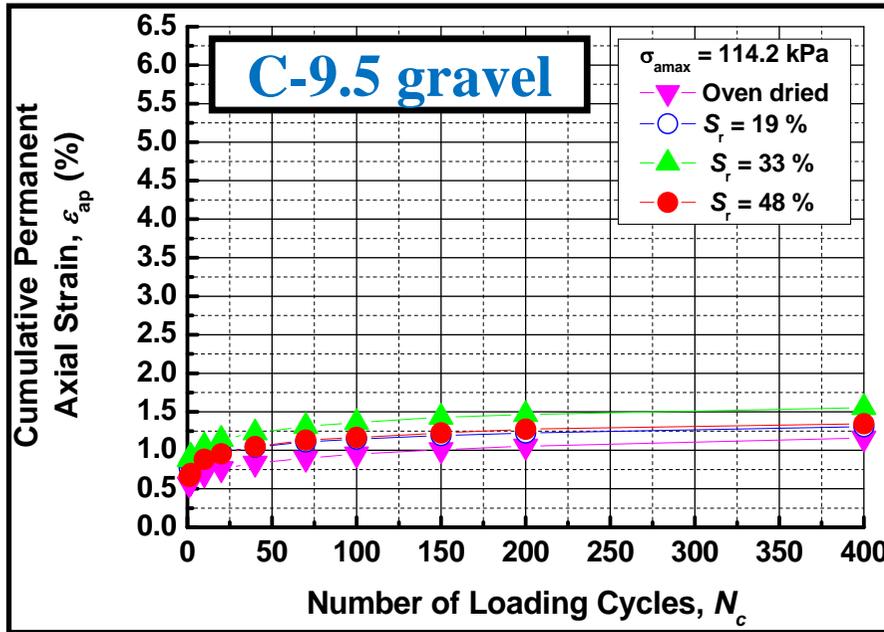
Stress – strain relations under different  $S_r$

Strain at same loading conditions under different  $S_r$

- Under same loading conditions, oven-dried sample has minimum shear and axial strains, thus indicating that it has maximum shear strength and stiffness.
- Wetting causes the sample to decrease the shear strength and stiffness, thereby increasing shear and axial strains.

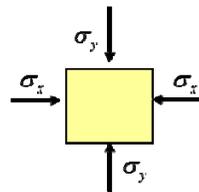
# Results of cyclic loading tests (1)

## Influence of water content & loading method



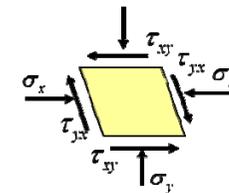
### Fixed-point Loading

Increase & decrease of loads



### Moving-wheel Loading

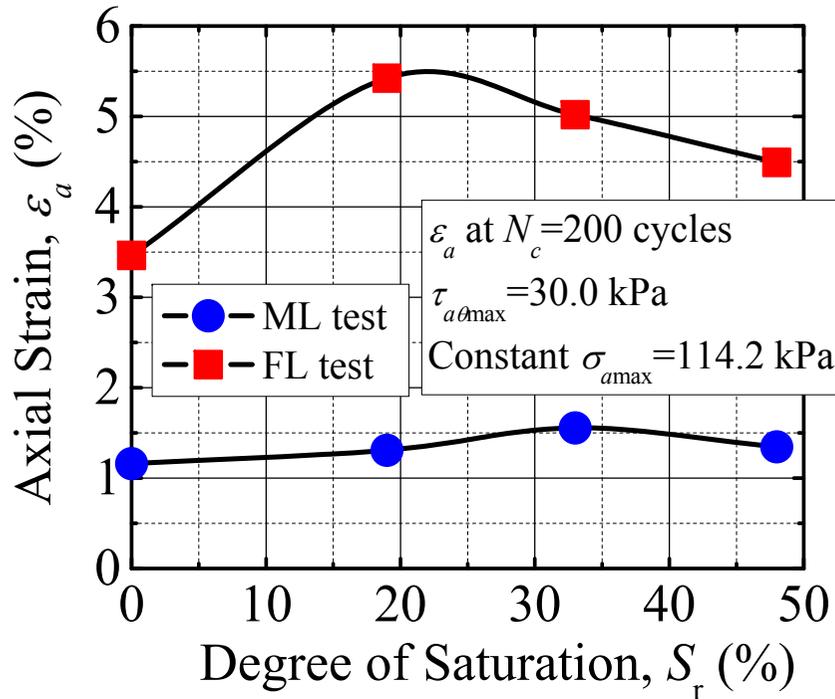
Increase & decrease of loads with principal stress axis rotation



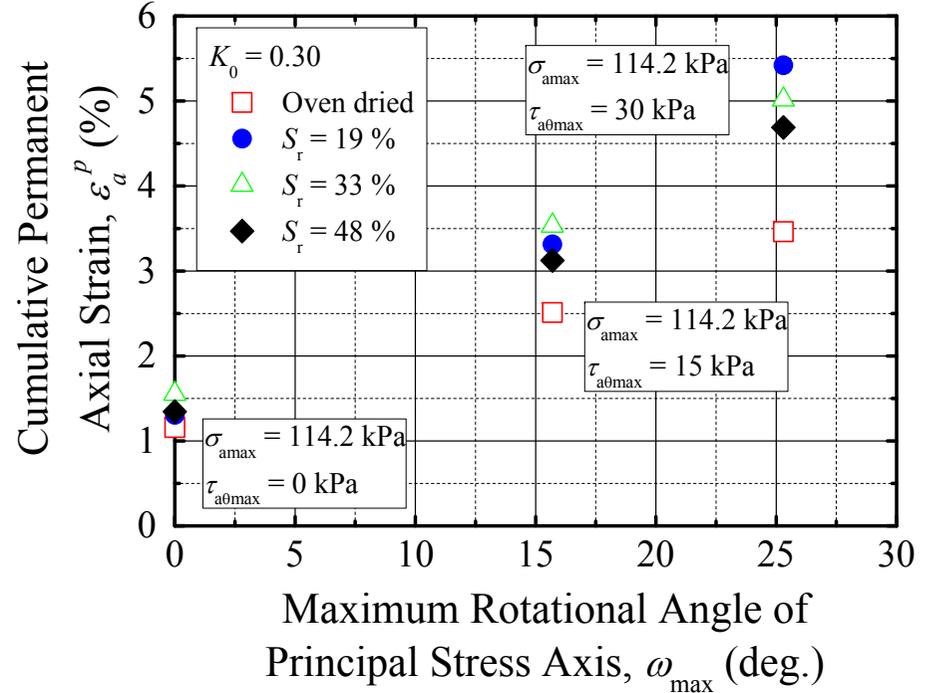
- Cyclic plastic deformation of wet gravel is more likely to increase with loading cycles than that of oven-dried gravel.
- The influence of water content on the cyclic plastic deformation for gravel appears more clearly in employing moving wheel loads.

# Results of cyclic loading tests (2)

## Influence of water content & loading method



Cyclic plastic deformation under different loading conditions and  $S_r$



Influences of principal stress axis rotation on cyclic plastic deformation

- Wetting causes the sample to decrease the shear strength and stiffness, thereby increasing cumulative permanent strain during cyclic loading.
- When rotational angle of principal stress axis increases with the increase in shear stress amplitude, cumulative permanent axial strain also increases.

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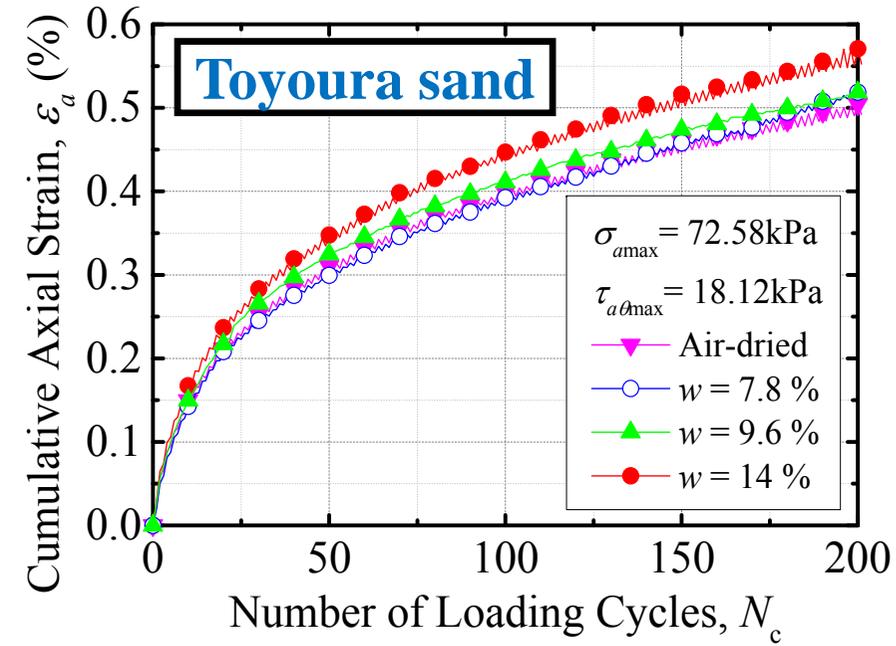
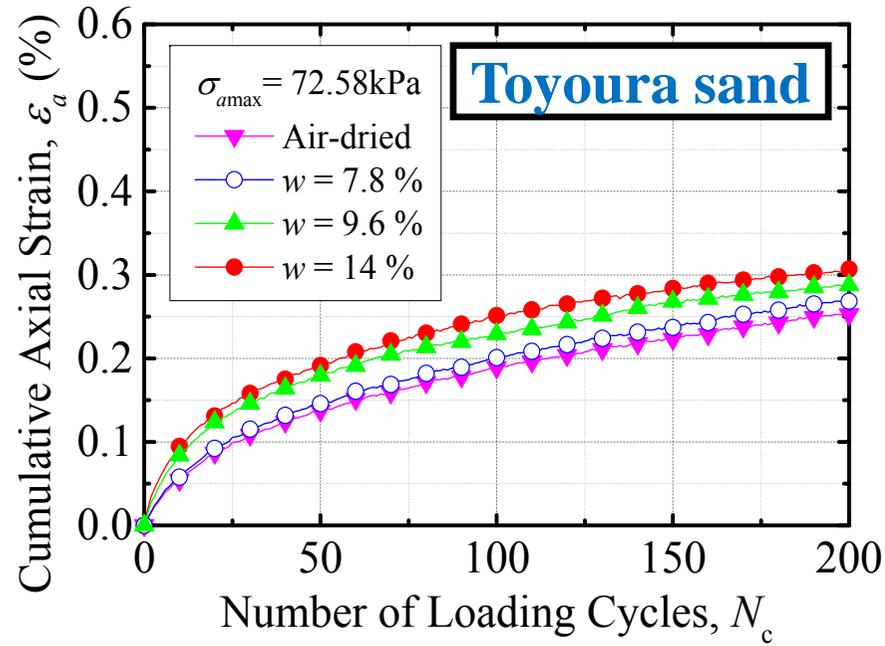
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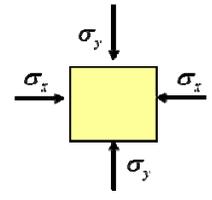
# Results of cyclic loading tests (1)

## Influence of water content & loading method



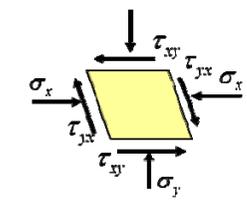
### Fixed-point Loading

Increase & decrease of loads



### Moving-wheel Loading

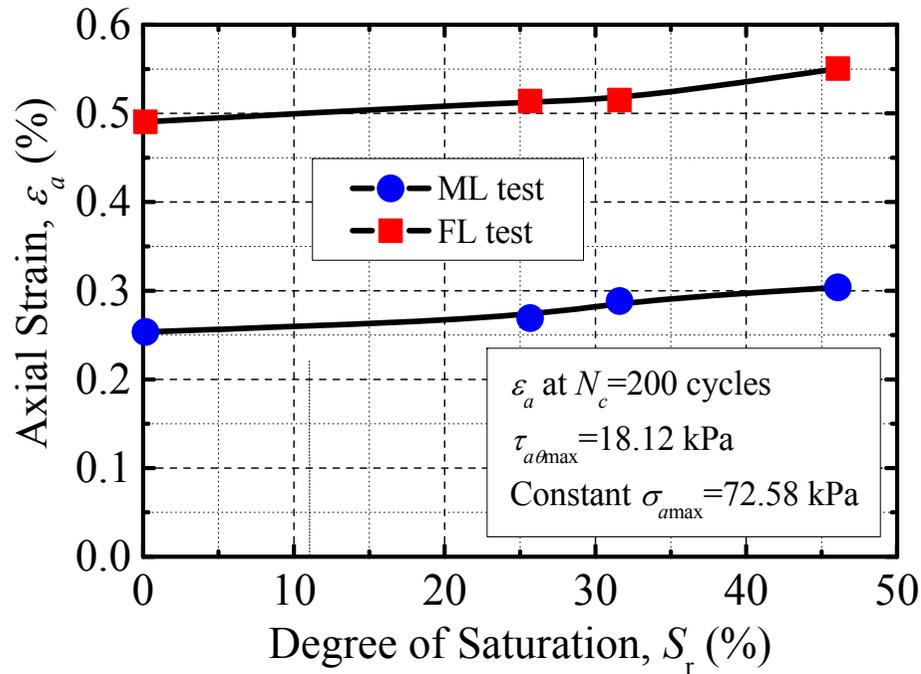
Increase & decrease of loads with principal stress axis rotation



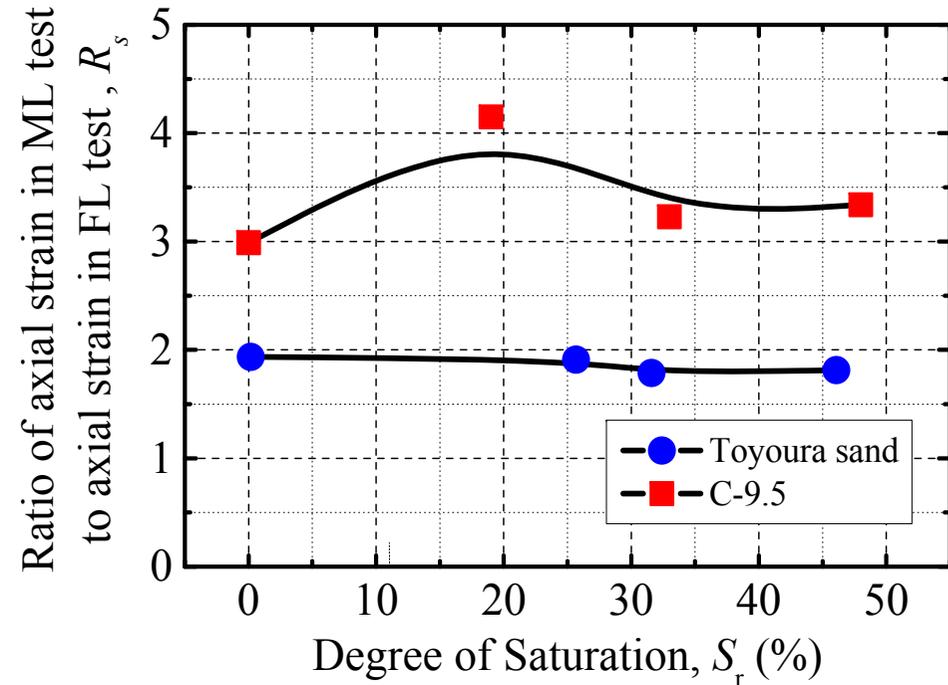
- As seen in the comparison for gravel, cyclic plastic deformation of sand in ML tests is larger than that in FL tests.
- Cyclic plastic deformation of wet sand is more likely to increase with loading cycles than that of air-dried sand.

# Results of cyclic loading tests (2)

## Influence of water content & loading method



Cyclic plastic deformation under different loading conditions and  $S_r$



Influences of water content on cyclic plastic deformation of gravel and sand

- Wetting of sand increases cumulative permanent strain during cyclic loading, regardless of loading method.
- Effects of water content on cyclic plastic deformation under moving-wheel loads differ depending on soil types.

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# Testing Method

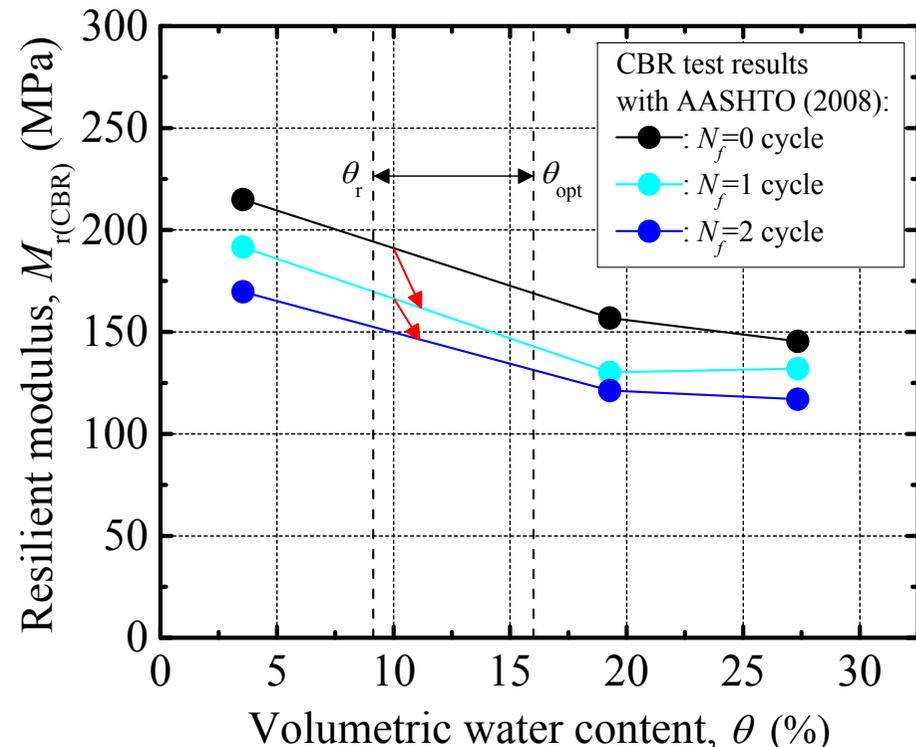
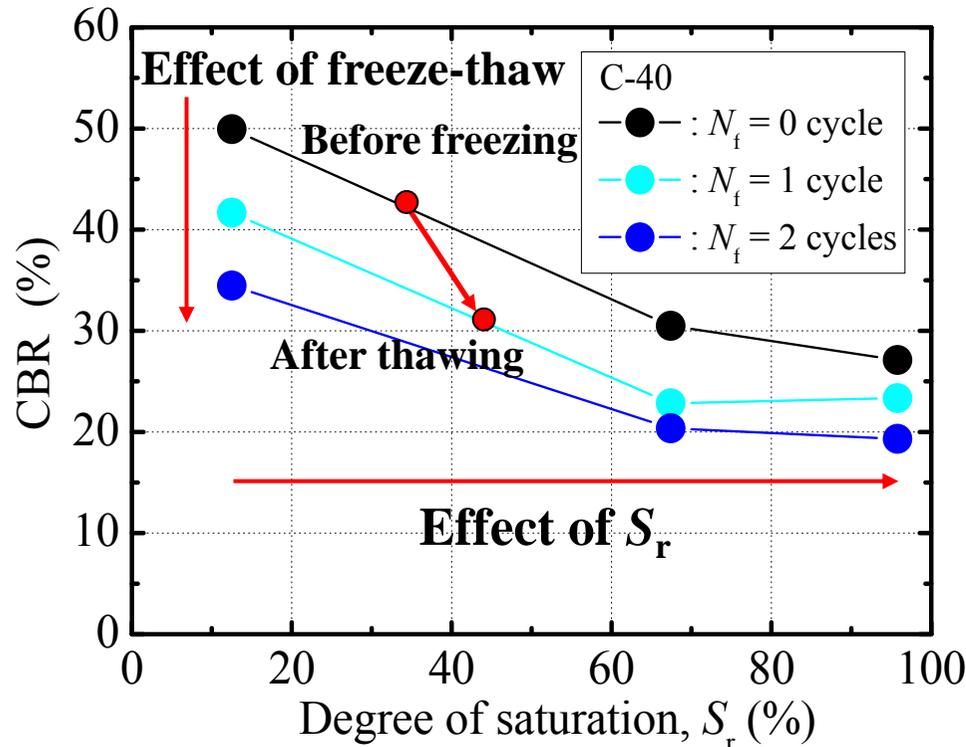
- Specimens in freeze-thaw CBR tests were prepared almost in the same manner as specimens in triaxial tests, except that the wet ( $S_r=67.6\%$ ) specimen was prepared by gravity drainage of saturated specimen.
- Freeze-thaw tests under the following experimental conditions, that is various freeze-thaw histories and water contents, were conducted in closed-system without water supply and drainage.
- After the freeze-thaw process, CBR tests for air-dried, wet and saturated specimens were carried out.
- Based on results of frost heaving tests, frost-susceptibility of natural crusher-run is low, regardless of freeze-thaw history and water content.

## Experimental conditions of freeze-thaw CBR tests

Name	Initial $S_r$	Initial $w$	Initial $\theta$	Freeze-thaw history
air-dried	8.2 %	1.2 %	2.3 %	0 (no freeze-thaw), 1, 2 cycle
wet	67.6 %	9.8 %	19.3 %	0 (no freeze-thaw), 1, 2 cycle
saturated	100 %	14.3 %	28.2 %	0 (no freeze-thaw), 1, 2 cycle

Note : under optimum water content ( $w=8.2\%$ ),  $S_r=57.0\%$  and  $\theta=16.2\%$ . 37

# Results of freeze-thaw CBR tests



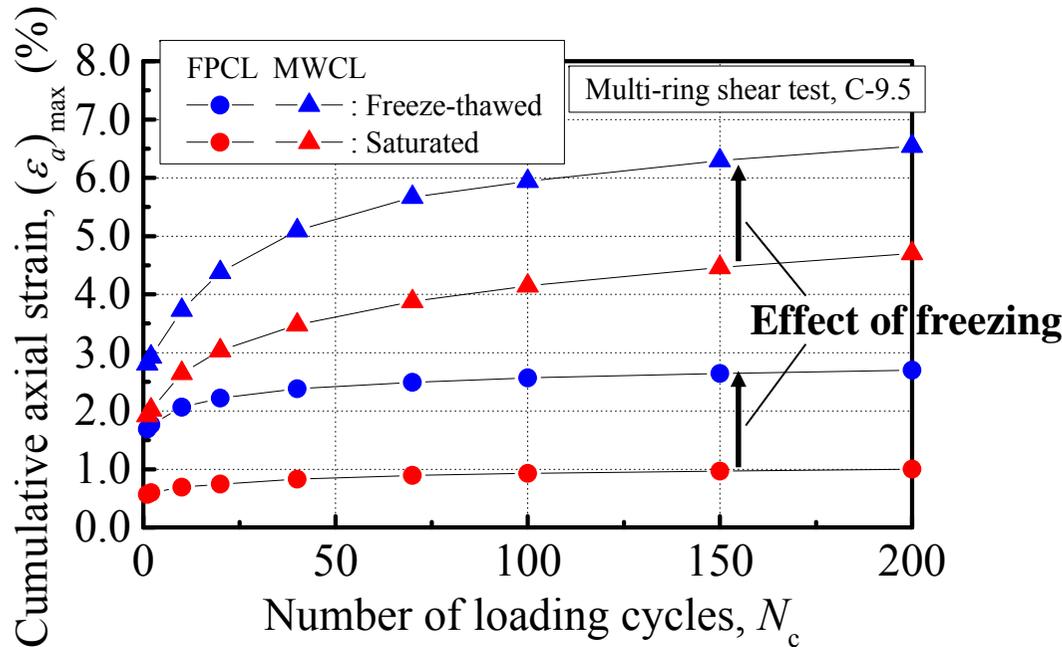
CBR under different freeze-thaw histories &  $S_r$

$M_r$  estimated using  $M_r = 17.6 \text{ CBR}^{0.64}$

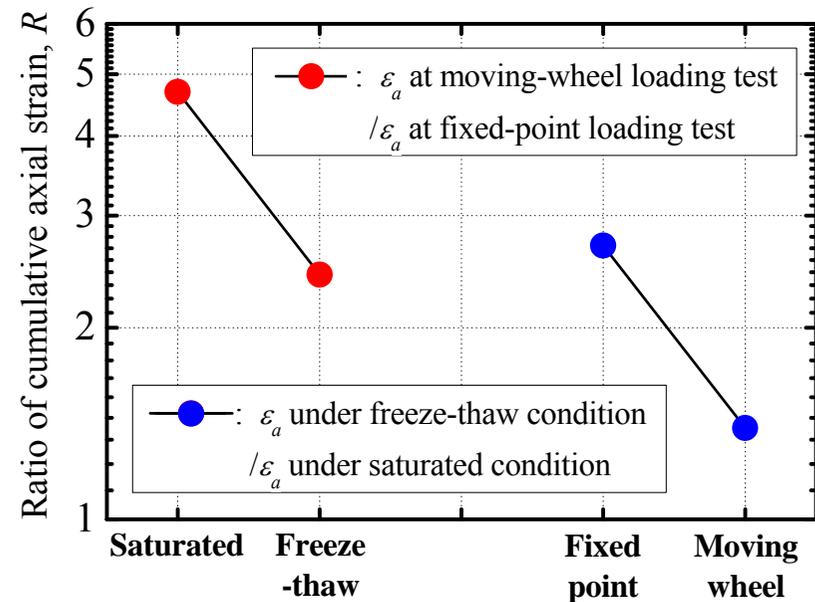
- CBR and  $M_r$  decrease with the increase in water content irrespective of the freeze-thaw history, and they drops with an increase in the freeze-thaw process cycles regardless of the water content.
- Considering the range of water content obtained from field measurement, it is expected that the freeze-thaw action seriously influences the bearing-capacity of unbound granular base course materials.

# Results of cyclic loading tests

## Influence of freeze-thaw & loading method



Cyclic plastic deformation under different freeze-thaw history



Effects of freeze-thaw on cyclic plastic deformation

- Freeze-thaw makes cumulative permanent strain during cyclic loading increase further in addition to the effect of water content.
- The influence of freeze-thaw on the cyclic plastic deformation should be considered even in a non-frost-susceptible material.

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# Proposal of Experimental Formula (1)

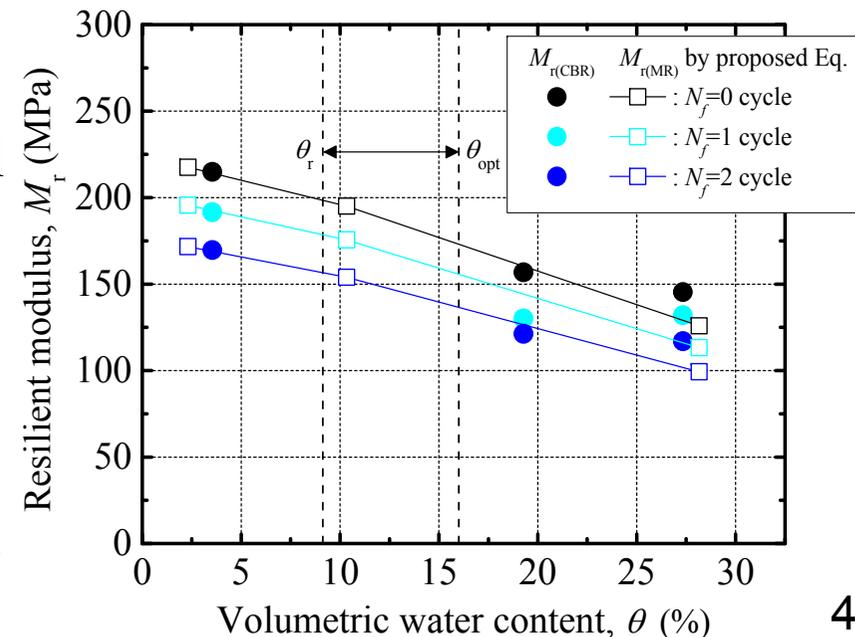
## Assumptions

- Effects of freeze-thaw actions can be expressed by a compensation function  $f(N_f, \theta)$ , which uses number of freeze-thaw process cycles and volumetric water content as explanatory variables.
- $M_r$  for C-40 subjected to repeated freeze-thaw actions can be estimated using the “modified universal model”, which is the universal model multiplied by the compensation function.

$$\text{Modified Universal model} : M_r = f(N_f, \theta) \cdot k_1 p_a \left( \frac{\sigma'_{ii}}{p_a} \right)^{k_2} \left( \frac{\tau_{oct}}{p_a} + 1 \right)^{k_3}$$

## Applicability of proposed formula

- Resilient modulus  $M_r$  estimated by proposed model seems to reproduce overall tendency for the  $M_r$  to decrease with the increase in the water content and the number of freeze-thaw process cycles.
- “Modified universal model” is sufficiently applicable to the quantitative evaluation of the  $M_r$  for unsaturated subbase course layer subjected to repeated freeze-thaw actions.



# Proposal of Experimental Formula (2)

## Assumptions

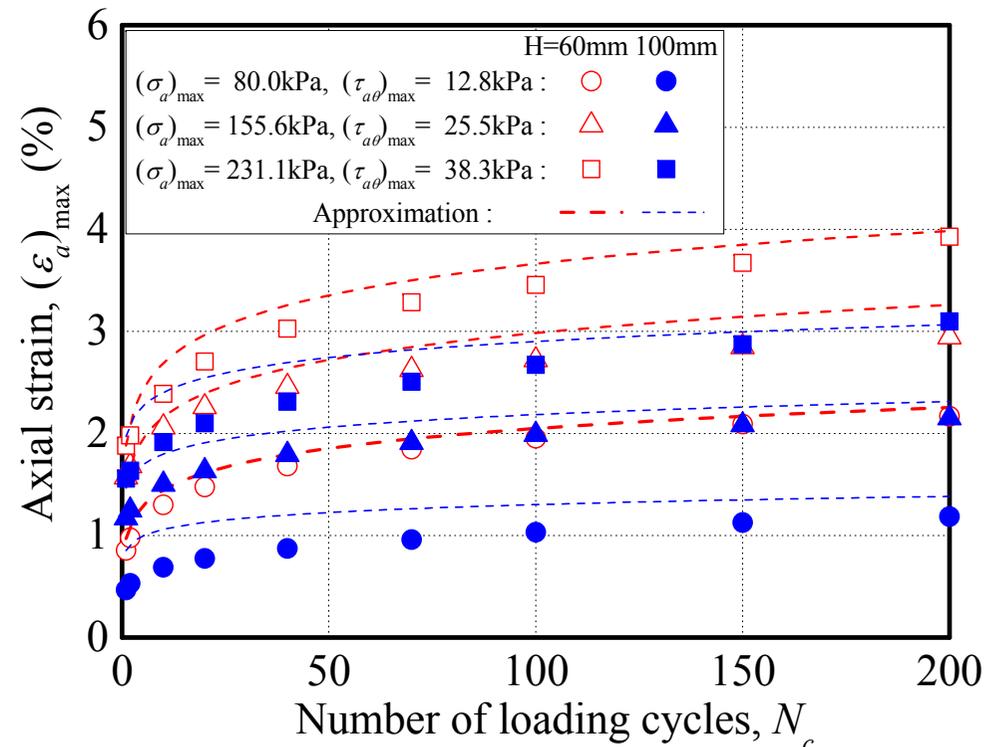
Under the same experimental conditions, cumulative axial strain in ML test can be estimated from cumulative axial strain of FL test by using average  $R_s$  as follows.

$$\varepsilon_a^{ML}(N_c) = R_{s\text{ave}} \cdot \varepsilon_a^{FL}(N_c) = \exp\left(a \frac{(\tau_{a\theta})_{\max}}{(\sigma_a)_{\max}}\right) \cdot \varepsilon_a^{FL}(N_c)$$

Note :  $a$  is a constant depending on water content.

## Applicability of proposed formula

- Proposed simple experimental formula is effective in estimating the cumulative strain characteristics of coarse granular materials in the principal stress axis rotation field.



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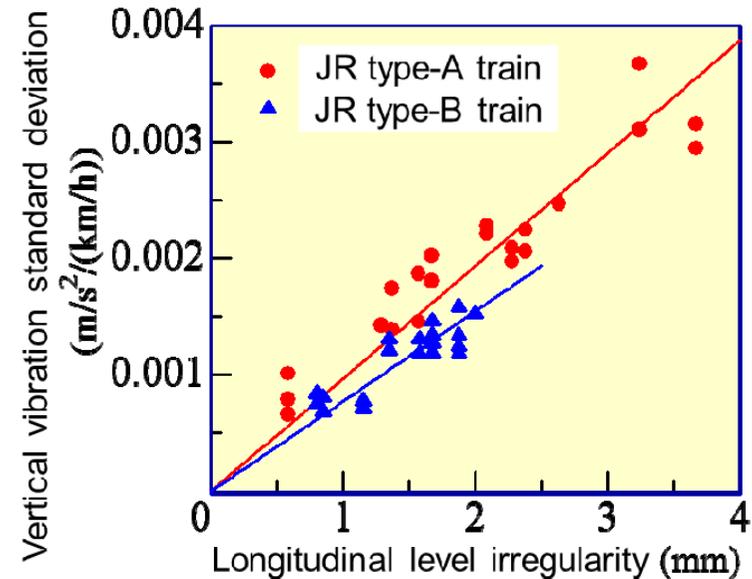


# Track condition of ballasted track

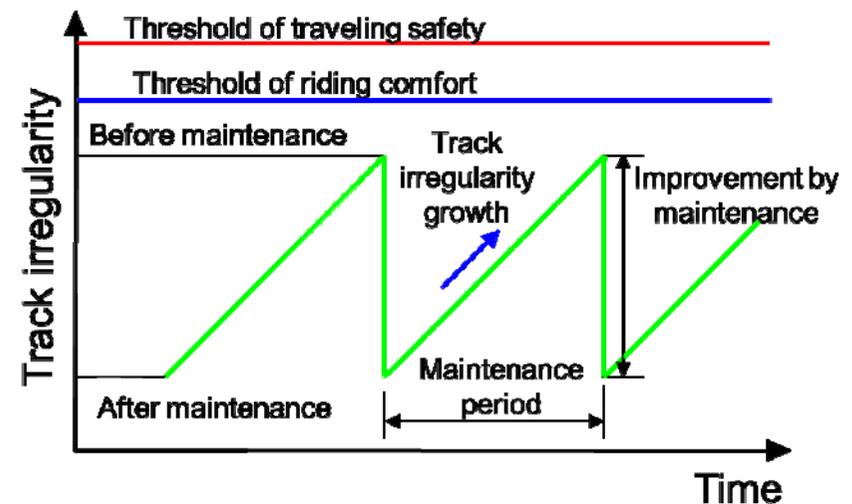
## Design criteria of ballasted track

- Train vibration should be smaller than threshold limit value.
  - Traveling safety
  - Riding comfort
- Train vibration increases along with the increase in track irregularity.
- Train vibration can be estimated by
 
$$\alpha = a \cdot V \cdot \sigma$$

$\alpha$ : train vibration  
 $V$ : train speed  
 $\sigma$ : track irregularity
- Precise prediction of track irregularity growth is important for rational design & maintenance works of ballasted track.



Train vibration and track irregularity



Change in track irregularity with time

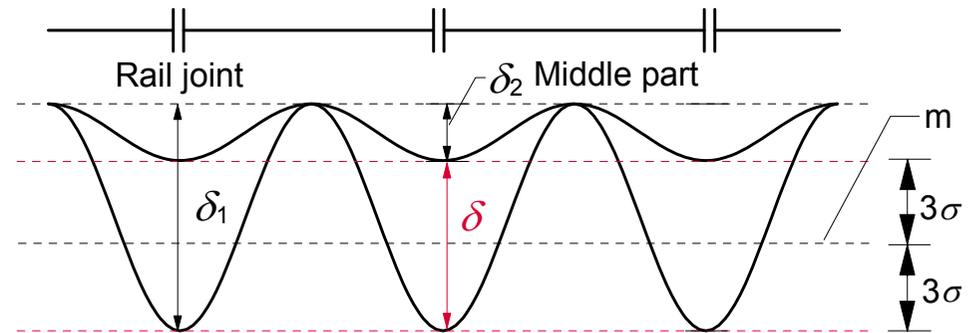
# Design procedures of ballasted track

**KEY POINTS** : How do we calculate track irregularity using settlement?

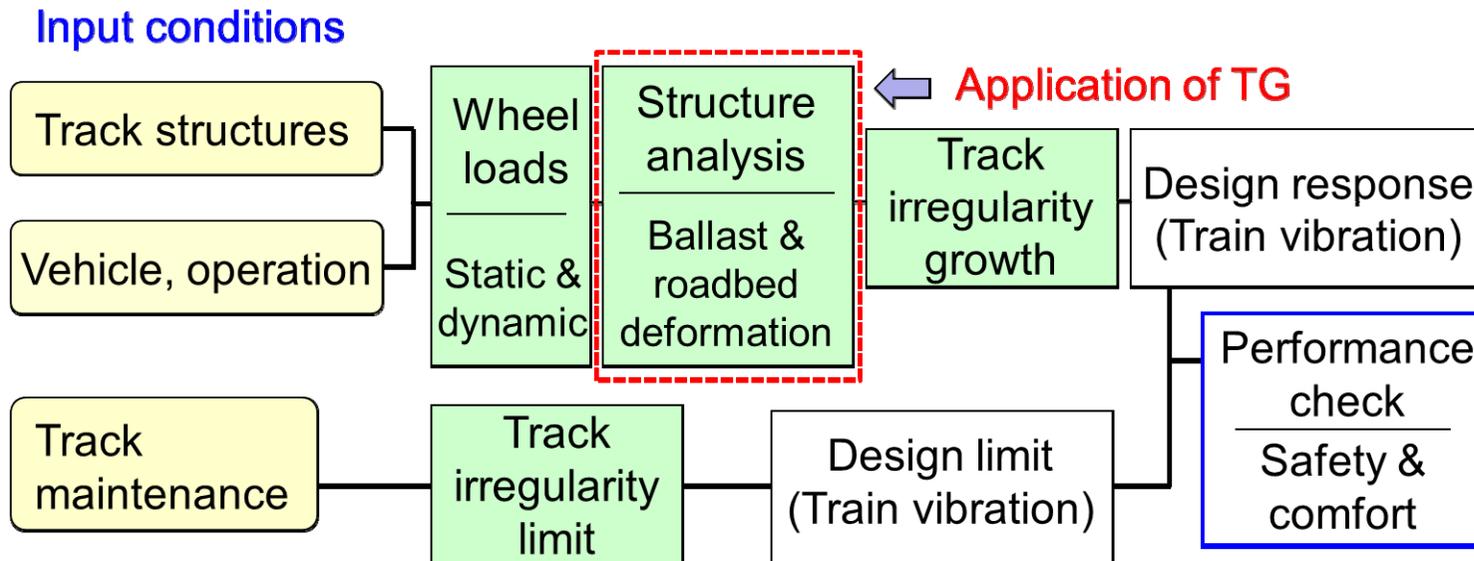
## Assumption

- Track irregularity is defined by the difference between rail settlement at rail joint and that at middle part.
- Standard deviation of track irregularity can be estimated by

$$\delta = m + 3\sigma = 6\sigma$$

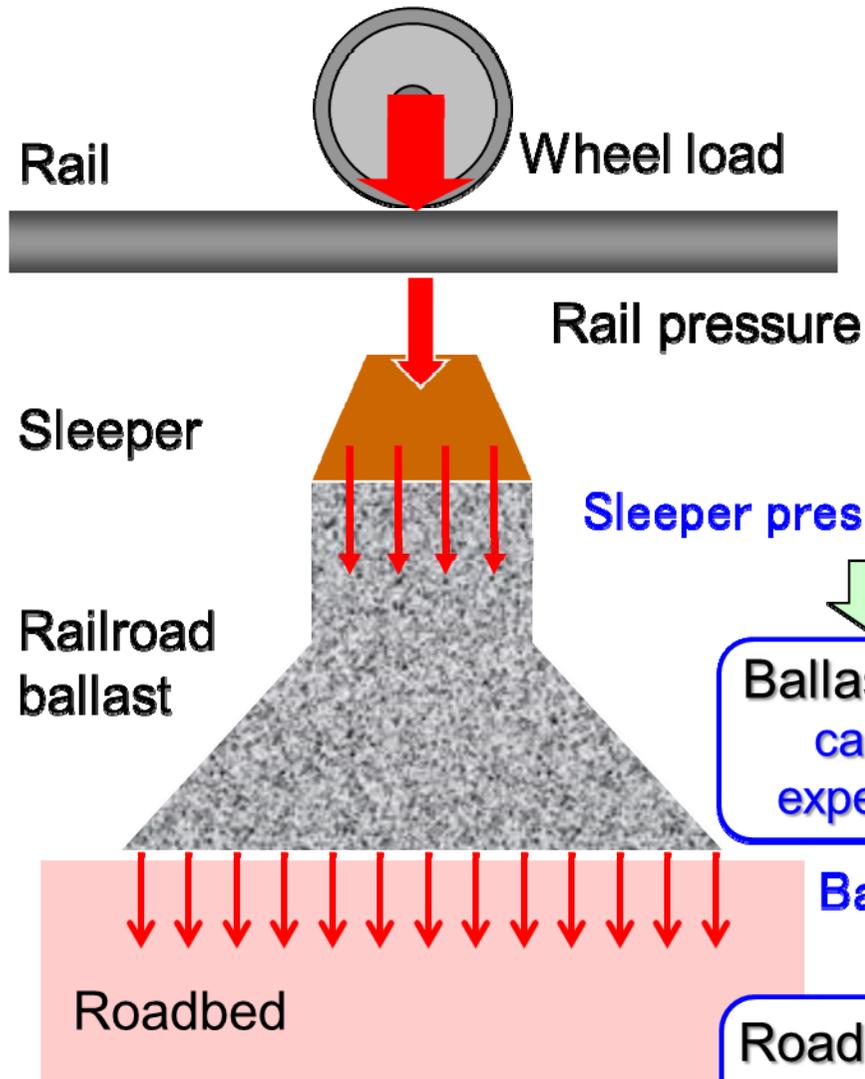


Rail settlement and track irregularity



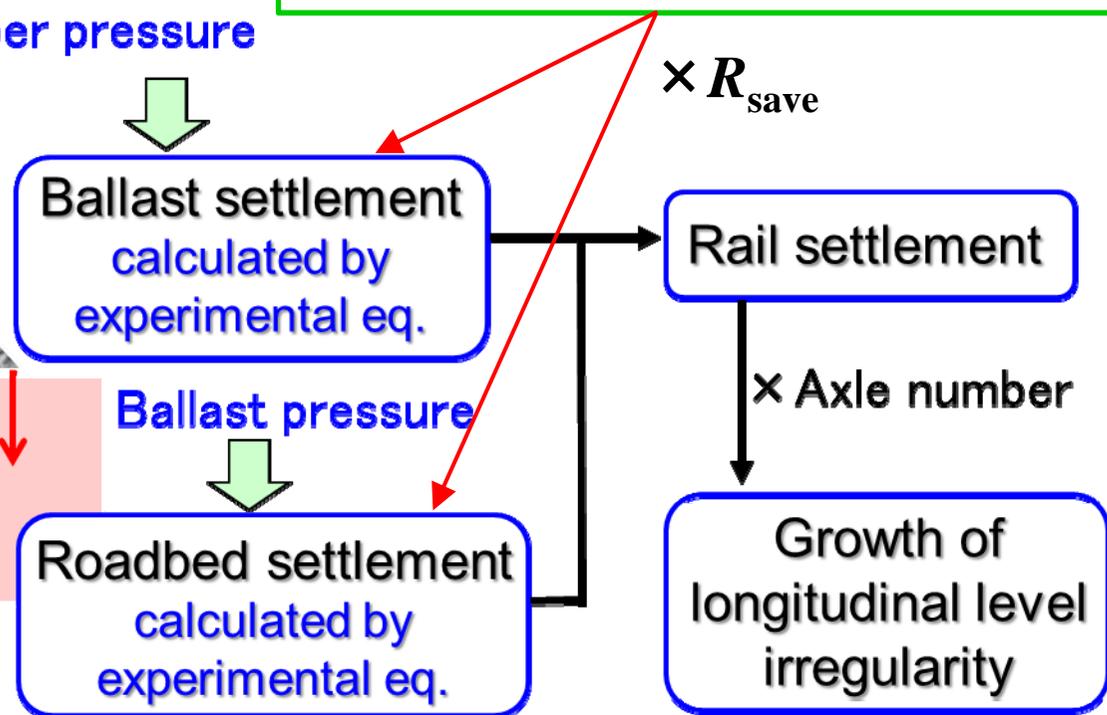
Flow of design procedures for ballasted track

# Application of advanced soil testing



## Key Point

- Results of advanced soil testing considering moving-wheel loads and water content change improve the calculation precision of rail settlement.



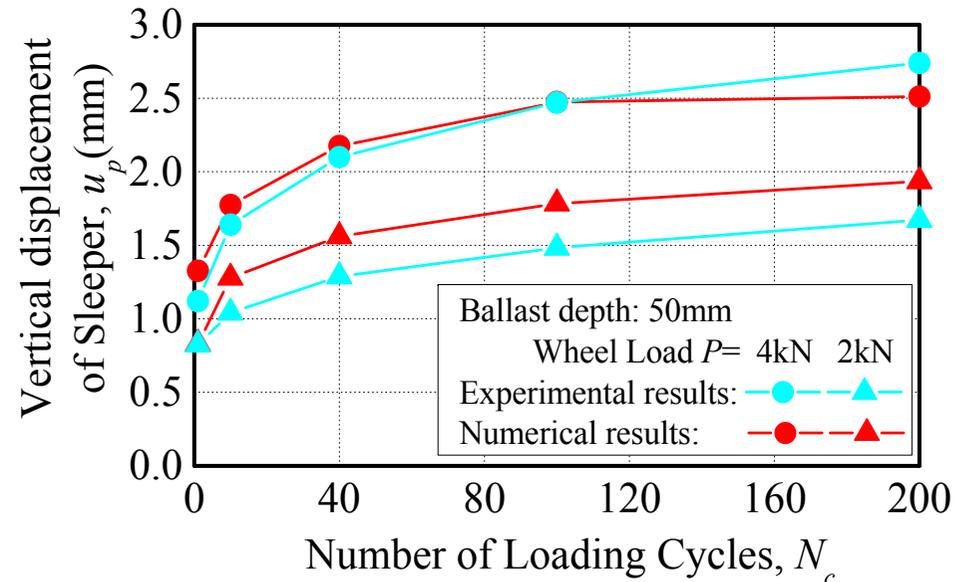
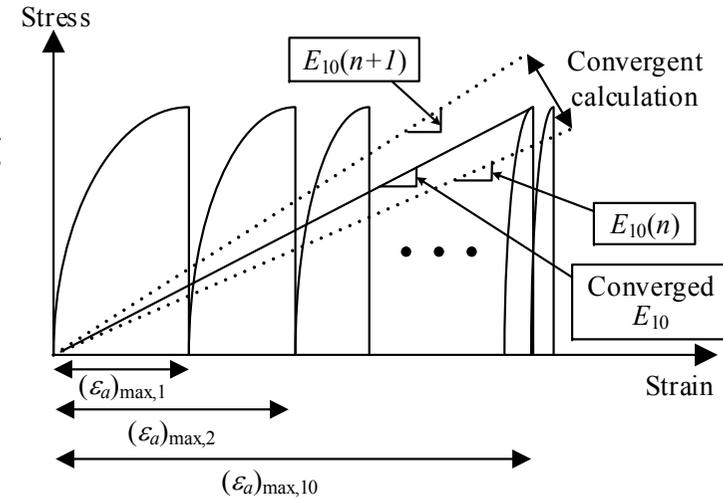
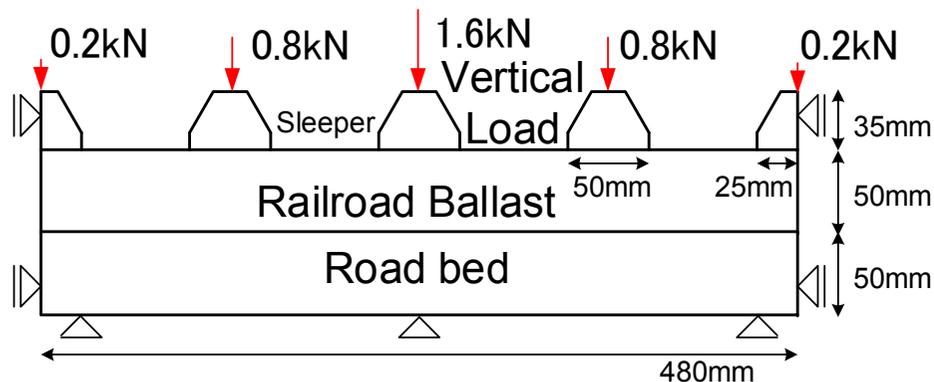
# FE analysis based on cumulative damage model

## Cumulative damage model

Deformation modulus of FE elements representing railroad ballast has been reduced in consideration of the cumulative strain characteristics of ballast, which is derived from advanced soil testing like multi-ring shear test.

## FE analysis of ballasted track

FE analysis based on cumulative damage model was conducted to simulate small scale model tests of ballasted track.



- If element tests are conducted under the experimental conditions similar to actual phenomena, even simple linear elastic FE analysis can roughly estimate cyclic plastic deformation of railroad ballast under repeated moving-wheel loads.

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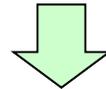
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# Concluding Remarks

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- Change in water content as well as the effect of freeze-thawing seriously influences the bearing capacity of coarse granular materials.
- The synergistic effects of principal stress axis rotation and water content strongly influence cyclic plastic deformation of coarse granular materials and roadbed materials.
- The loading rate has a strong influence on the deformation-strength characteristics of coarse granular materials in unsaturated condition.



- When developing a rational design standard of ballasted track in cold regions for precisely predicting the mechanical behavior and evaluating the long-term performance, it is important to give a special consideration to the influence of water content, freeze-thaw history, loading rate, and principal stress axis rotation on the deformation-strength characteristics.

*Thank You for Your Kind Attention*

