

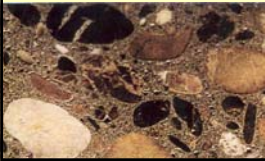
Corrosion of Building Materials

Prof. Dr. Andreas Gerdes

Concrete and Mortar

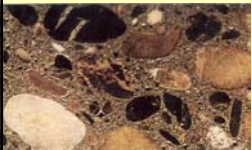
Concrete and mortar is a mixture of...

- Portland Cement
- Aggregates
- Water
- Admixtures

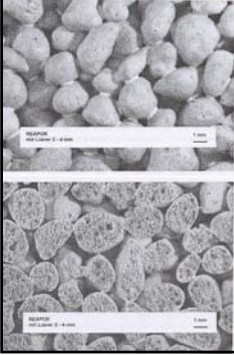


Aggregates I

Natural Aggregates	
Basalt	Mica
Gneiss	Opalit
Granite	Grauwacke
Quartz	Flint
Calcite	



Aggregates



Industrial Aggregates
Bims
Blähton
Gebälhte Schlacke
Gebälhte Asche
Gebälhtes Glas

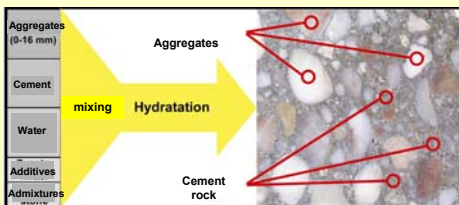
Aggregates III

Recycling-Aggregates
Excavation (Gotthard-tunnel)
FRANKA-Procedure
Crusher
Impact Mill



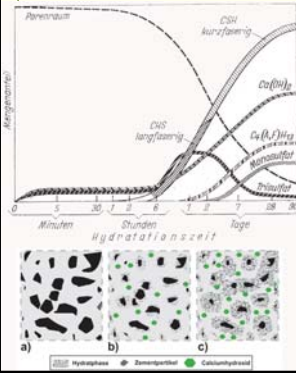
Properties of cement based materials

- Structure
- Chemical reactivity



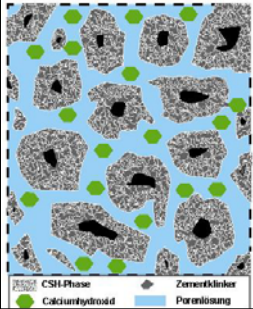
Structure build up due to hydration

- Formation of CSH-gel
- Decrease of pore volume
- Inclusion of calcium hydroxide in the pore volume / cement stone

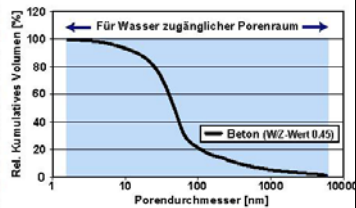


Structure of cement based materials

- Gel pores in CSH-Cluster
- Capillary pores between the Cluster



Structure of cement based materials



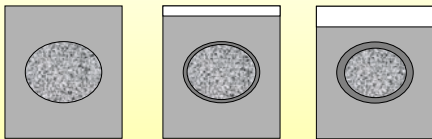
Pore classification

Concrete-technological procedure	Pore type		Pore classification by Setzer	Pore classification by IUPAC
compacting	Compacting pores	Large pores	> 2 mm	-
Air entraining agents	Air pores	Micro-capillaries	50 μm – 2 mm	50 μm – 2 mm
W/C (water-cement-ratio)	Capillary pores	Capillaries	2 μm – 50 μm	2 μm – 50 μm
		Micro-capillaries	5 nm - 2 μm	5 nm - 2 μm
Hydration and type of cement	Gel pores	Mesopores	2 nm – 50 nm	2 nm – 50 nm
		Micropores	< 2 nm	< 2 nm

Pore size and Pore size distribution



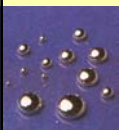
Measuring of total porosity and pore size distribution by mercury pressure porosimetry



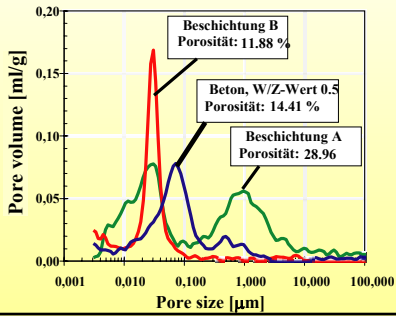
0 bar 1000 bar 2000 bar

Pressure is increased step by step up to 350 MPa

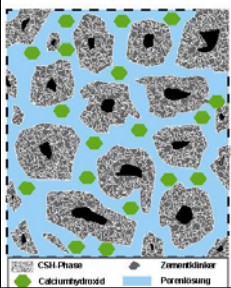
- Total mercury content is equivalent to total porosity
- With increasing pressure lower pore radii were filled



Pore size distribution of cement based materials



Composition of the pore solution

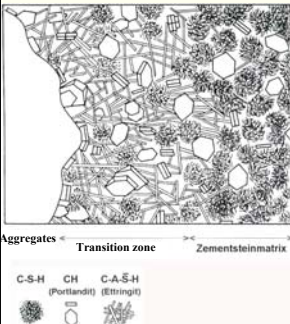


- Calcium hydroxide [Ca(OH)₂]
- Potassium hydroxide [KOH]
- Sodium hydroxide [NaOH]
- Calcium sulfate [CaSO₄]

Saturated Ca(OH)₂-solution:
pH 12.3

Pore solution of cement rock:
pH 12.3 – 13.0

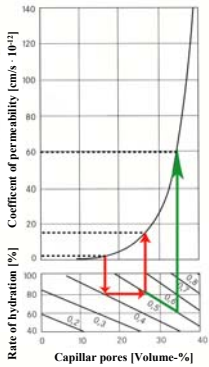
Microstructure of the phase boundary



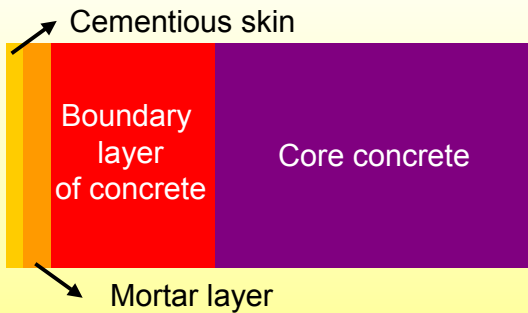
1. The phase boundary between aggregate and cement rock is 3.5 times more porous than the cement rock
2. Structure of the interface
 - Duplex film (1 µm thick)
 - Crossover zone (approx. 50 µm thick)
3. Reasons for low strength
 - high porosity (high W/C-ratio)
 - Low crystalline bond of portlandite crystals
 - Hadley grains of hydration products

Permeability of cement based materials

Researches made by
POWER's already made in the
1950's!!

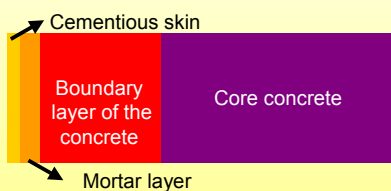


The boundary layer of concrete



Influencing factors on the permeability of the boundary layer

- W/C-ratio
- Treatment after construction
- Used cement



Alkali-Silica-Reaction

1920 → Verification of a reaction between aggregates and alkaline chemicals

1965 → no danger in Germany
"Lachswehr"-bridge: built in 1965/66 , demolished in 1968



Mechanismen is well-known since the 1970's

Alkali-Silica-Reaction (ASR)

1. Aggregate Reactivity

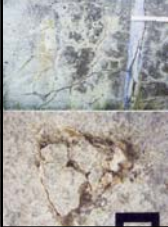
- Interferences in the grid structure
- Temperature
- Aggregate size
- pH-value of the solution

2. Influences on ASR

- Environment
 - Temperature
 - Humidity
- Amount and size of aggregates
- Permeability of the concrete
- Alkaline content of the cement
- Cement content

3. Counteractive measures

- Bond of alkaline content by pozzolanes
- Water repellent treatment



Damage processes Carbonation



Carbonation of reinforced concrete

Brief historical abstract



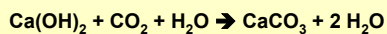
- 1879 Cement cover prevents iron from oxidizing (rust)
- 1908 Realisation that the alkaline ambience prevents this corrosion
- 1916 „Corrosion of highly importance because of security reasons “
- 1919 1.5 cm cover concrete shall be enough to prevent corrosion of the reinforcement

Carbonation of reinforced concrete

The reaction of the CSH-phases with CO₂ is called **CARBONATION**



The Ca(OH)₂ (approx. 20 mass-%) which is build up by the hydration of cement and KOH & NaOH in the pore solution are responsible for the pH of approx. 12.3 to 13 of the concrete.



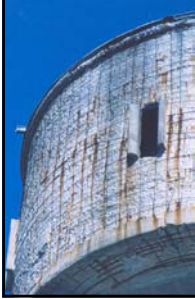
Reasons for damages by carbonation

- Low cement content
- Minor dimension of cover concrete
- CO₂-content of the air
- Additional pollutants (NO, NO₂, NO₃)
- Insufficient manufacture of th concrete

Reaction steps of the carbonation

Carbonation is a coupled process:
Transport & chemical reaction

1. Diffusion of CO_2 into the cement rock
2. Solution of CO_2 in the pore solution
 $\text{CO}_2 + \text{H}_2\text{O} \rightarrow \text{H}_2\text{CO}_3 \rightarrow 2 \text{H}^+ + \text{CO}_3^{2-}$
3. Reaction of Ca(OH)_2 with H_2CO_3
 $\text{Ca(OH)}_2 + \text{H}_2\text{CO}_3 \rightarrow \text{CaCO}_3 + 2 \text{H}_2\text{O}$
Carbonation of the alkaline hydroxides
 $2 \text{NaOH} + \text{CO}_2 \rightarrow \text{Na}_2\text{CO}_3 + \text{H}_2\text{O}$
 $\text{Na}_2\text{CO}_3 + \text{Ca(OH)}_2 \rightarrow \text{CaCO}_3 + 2\text{NaOH}$
4. Decomposition of cement rock
 $\text{C}_x\text{SH}_y + x\text{CO}_2 \rightarrow x\text{CaCO}_3 + \text{SiO}_2 \cdot y \text{H}_2\text{O}$



Consequences of carbonation

CARBONATION has positive & negative effects on reinforced concrete



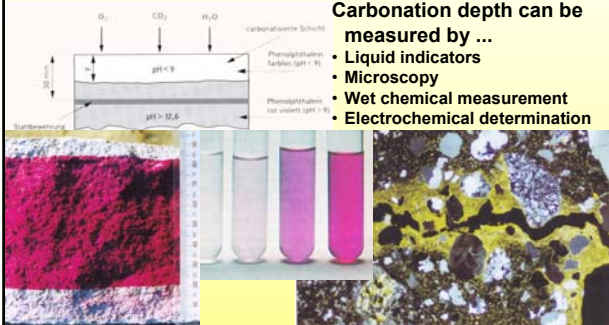
POSITIVE:
Increase in density of the concrete
($\Delta V=11\%$)

NEGATIVE:
Decomposition of the protective layer
(approx. 50 nm) consisting of iron hydroxides and oxides on the surface of the reinforcement
→ Corrosion of the reinforcement

Determination of the depth of Carbonation

Carbonation depth can be measured by ...

- Liquid indicators
- Microscopy
- Wet chemical measurement
- Electrochemical determination



Calculation of carbonation depth

Carbonation is a diffusion controlled process:

1. Fick's law

Mass balance

$$\frac{dm}{dt} = D \cdot F \cdot \frac{dc}{dy}$$

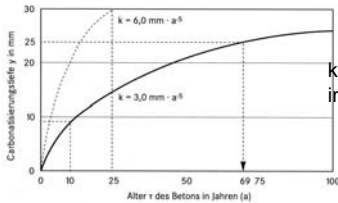
$$dm = m_0 \cdot F \cdot dy$$

m	=	CO ₂ mass transported through the concrete surface [kg]
t	=	Time of admission [a]
D	=	Diffusion coefficient [m ² /s]
c	=	CO ₂ concentration in the air and in the pore structure [kg/m ³]
y	=	Thickness of carbonated layer [m]
m₀	=	Absorbed CO ₂ mass per volume unit of concrete [kg]
F	=	Area of carbonating concrete [m ²]

Calculation of carbonation depth – √t-law

Solution of the diffusion equation:

$$y = \sqrt{\frac{2 \cdot D \cdot c_0}{m_0}} \cdot \sqrt{t}$$



$$y = k \cdot \sqrt{t}$$

k = Carbonation coefficient
in mm/a^{0.5}

Factors affecting the depth of carbonation



- Concentration of CO₂
- Humidity
- Concrete quality (W/C-ratio)
- Type of cement
- aftercare
- Aggregates, additives, admixtures
- Temperature

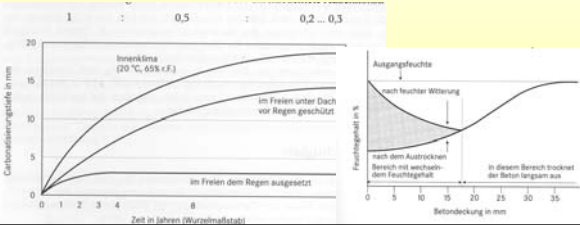
Factors affecting the depth of carbonation – CO₂-concentration

- CO₂ content in air is approx. 0.03% and almost constant.
- Higher contents are a result of the type of use (e.g. basement garages or wine cellars)



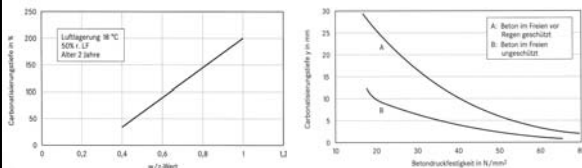
Factors affecting the depth of carbonation - Humidity

- High moisture content: lowers diffusion ($D \ll$)
- Low moisture content: lowers reaction ($RG \ll$)
- ➔ Optimal value lies between 60% and 80% rel. humidity



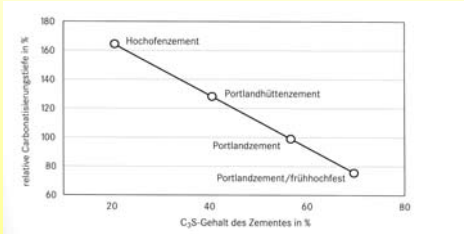
Factors affecting the depth of carbonation – W/C-ratio

- The Depth of carbonation is mainly determined by the permeability of the boundary concrete
- ➔ W/C-ratio
- Depending on the W/C-ratio different final depth of carbonation can be detected



Factors affecting the depth of carbonation- Type of cement

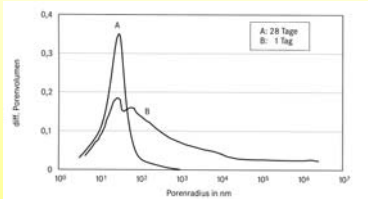
- Depending on the cement type different contents of $\text{Ca}(\text{OH})_2$ were build up and structure of different density were formed



Factors affecting the depth of carbonation - Aftercare

- Adequate aftercare forms a denser concrete boundary layer
- Aftercare by keeping in the formwork, covering with foils or water storage meadows, aftercare additives, spraying with water

→ Increasing the carbonation resistance



Factors affecting the depth of carbonation - Aggregates, additives and admixtures



- Low weight aggregates (Perlite) result in an easier transport of water and CO_2
- Effect of admixtures is unexplained
- Additives form a denser concrete boundary layer; addition of up to 40% show no problem in the reaction with alkali

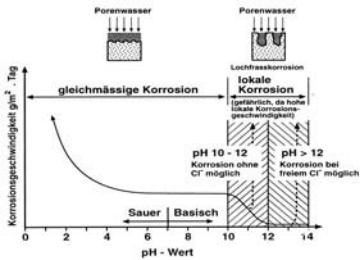
Corrosion of the reinforcement

Contrary to steel kept at the atmosphere a thin PASSIVE LAYER is formed in high alkaline medium
→ Low speed of corrosion



Destabilisation of the passive layer

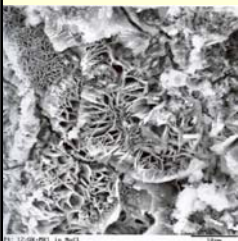
Korrosionsverhalten von Stahl in Beton



Due to exposure of chlorides and carbonation the thin passive layer is regional destroyed
→ Depassivation

Corrosion of the reinforcement

Conditions for reinforcement corrosion



- Availability of an electrolyte
- „Break-down“ of the passive layer
- Adequate oxygen supply
- Formation of local potentials

Phase of Initiation

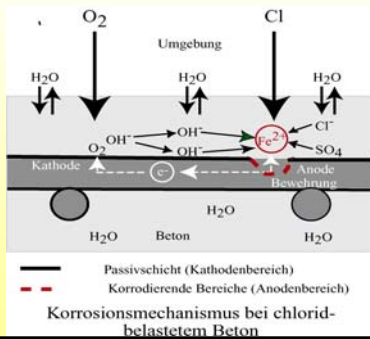
During the initiation phase aggressive compounds penetrate the boundary layer of concrete.

→ The period of initiation is determined by:

- ... Thickness of concrete cover
- ... Penetration speed of pollutants
- ... Concentration of pollutants

→ Calculation made by the civil engineer

Speed of corrosion



Characterising the process of corrosion

The process of corrosion can be expressed by the rate of degradation.

For example: 10-1000 μm per year

Methods to determine the corrosion

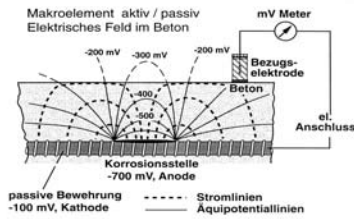
- Potential measurement ...
- Electrical resistance of the concrete
- Impedance measurement

... Allows fundamental evidence of

- Location and state of areas of corrosion
- Actual state of the corrosion of the reinforcement

Corrosion potential of the reinforcement

Due to the current flow induced by the corrosion an electrical field is build up. The **potential of the corrosion** is representative for the **actual state of the corrosion**.



Prinzip der Potentialmessung auf der Betonoberfläche zum Auffinden korrodierender Bereiche der Bewehrung

Phase II: Potential measurement



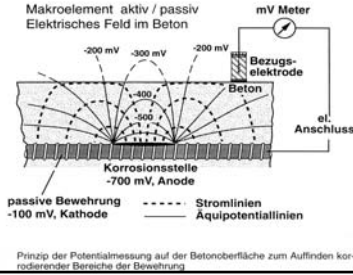
- After removal of the surface
- Up to 1000 m²/d



- In complex buildings
- Up to 1000 points/d

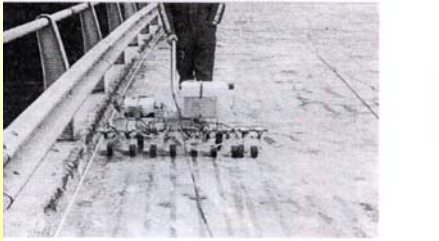
Implementation of the measurement

- Connecting the Cu/CuSO₄-electrode with volt meter
- Connecting reinforcement with volt meter
- Measurement of the corrosion potential



Performance of the measurement

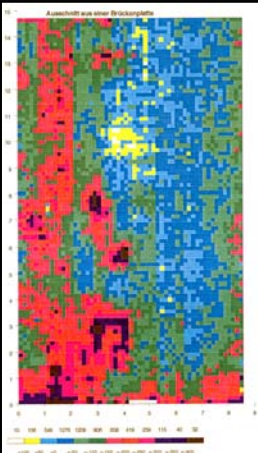
Large areas can be scanned by wheel-electrodes. The results are evaluated by computers.



Das im Rahmen des Forschungsprojekts entwickelte IBWK-Messsystem für Potentialfeldmessungen mit acht Radelektroden im praktischen Einsatz

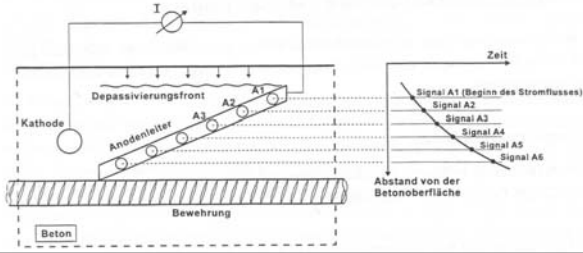
Typical results

Analysis is performed by computers. The output is given as „potential mappings“.



Test probe for monitoring

Assembly of the test probes in very exposed concrete members.



Condition analysis of a balcony plate

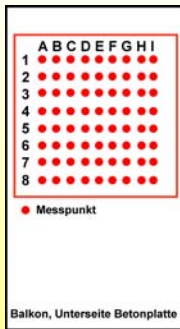
Balcony – State of repair



Balcony – State of repair



Results of balcony analysis



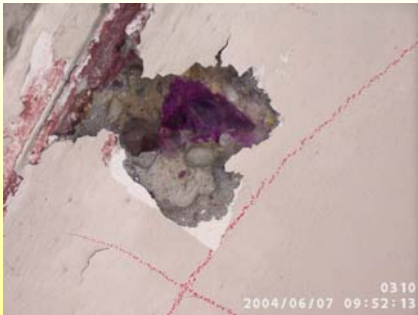
Results of balcony analysis



Results of balcony analysis



Results of balcony analysis



Results of balcony analysis

Adlerrasse Nr. 38 - 5. OG

Bewehrungsüberdeckung in mm										
A	B	C	D	E	F	G	H	I	Mittelwert	Standardabw.
19	23	13	25	33	25	31	29	10	23,9	5,0
9	11	27	12	18	24	21	7	5	14,9	7,9
15	18	21	21	28	25	11	-	-	19,9	5,8
19	20	14	9	9	14	18	8	-	13,9	4,9
18	20	14	8	9	14	18	8	-	13,6	4,8
10	8	8	7	13	14	16	22	-	12,3	5,1
9	9	12	29	13	30	28	9	-	17,4	8,8
9	8	5	9	7	22	30	32	11	14,8	10,4
Mittelwert Bewehrungsüberdeckung/Standardabweichung:									16,2	7,1

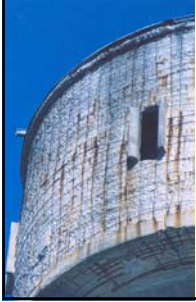
Carbonatisierungstiefe			
Bohrkern 1	Bohrkern 2	Bohrkern 3	Mittelwert Carbonatisierungstiefe
15	20	12	15,7

Results of balcony analysis

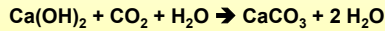
Position	Steinstrasse Nr. 19 - 5. OG										Mittelwert	Standardabw.
	A	B	C	D	E	G	H	I	J	K		
1	37	22	26	27	28	22	21	17	-	-	24,9	6,0
2	41	33	29	22	37	26	19	-	-	-	29,6	8,8
3	34	17	34	34	23	12	19	-	-	-	24,7	8,3
4	38	11	32	34	32	32	22	-	-	-	28,3	8,7
5	37	34	28	29	34	26	-	-	-	-	29,8	6,3
6	36	36	33	26	17	23	-	-	-	-	28,6	7,8
7												
8												
Mittelwert Bewehrungsüberdeckung/Standardabweichung:											27,6	7,9
Carbonatierungstiefe												
Bohrkern 1	Bohrkern 2	Bohrkern 3	Mittelwert	Carbonatierungstiefe								
28	28	15		18,3								

Carbonation of reinforced concrete

The reaction of the CSH-phases with CO₂ is called
CARBONATION



The Ca(OH)₂ (approx. 20 mass-%) which is build up by the hydration of cement and KOH & NaOH in the pore solution are responsible for the pH of approx. 12.3 to 13 of the concrete.

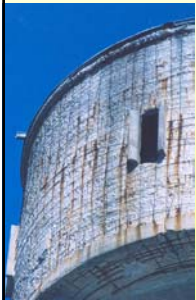


Reasons for damages by carbonation

- Low cement content
- Minor dimension of cover concrete
- CO₂-content of the air
- Additional pollutants (NO, NO₂, NO₃)
- Insufficient manufacture of th concrete

Reaction steps of the carbonation

Carbonation is a coupled process:
Transport & chemical reaction



1. Diffusion of CO₂ into the cement rock

2. Solution of CO₂ in the pore solution
 $\text{CO}_2 + \text{H}_2\text{O} \rightarrow \text{H}_2\text{CO}_3 \rightarrow 2 \text{H}^+ + \text{CO}_3^{2-}$

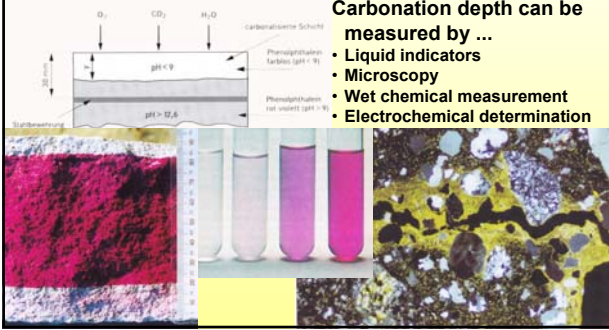
3. Reaction of Ca(OH)₂ with H₂CO₃
 $\text{Ca(OH)}_2 + \text{H}_2\text{CO}_3 \rightarrow \text{CaCO}_3 + 2 \text{H}_2\text{O}$
Carbonation of the alkaline hydroxides
 $2 \text{NaOH} + \text{CO}_2 \rightarrow \text{Na}_2\text{CO}_3 + \text{H}_2\text{O}$
 $\text{Na}_2\text{CO}_3 + \text{Ca(OH)}_2 \rightarrow \text{CaCO}_3 + 2\text{NaOH}$

4. Decomposition of cement rock
 $\text{C}_x\text{SH}_y + x\text{CO}_2 \rightarrow x\text{CaCO}_3 + \text{SiO}_2 \cdot y\text{H}_2\text{O}$

Determination of the depth of Carbonation

Carbonation depth can be measured by ...

- Liquid indicators
- Microscopy
- Wet chemical measurement
- Electrochemical determination



Calculation of carbonation depth

Carbonation is a diffusion controlled process:

1. Fick's law

Mass balance

$$\frac{dm}{dt} = D \cdot F \cdot \frac{dc}{dy}$$

$$dm = m_0 \cdot F \cdot dy$$

m	= CO ₂ mass transported through the concrete surface [kg]
t	= Time of admission [a]
D	= Diffusion coefficient [m ² /s]
c	= CO ₂ concentration in the air and in the pore structure [kg/m ³]
y	= Thickness of carbonated layer [m]
m₀	= Absorbed CO ₂ mass per volume unit of concrete [kg]
F	= Area of carbonating concrete [m ²]

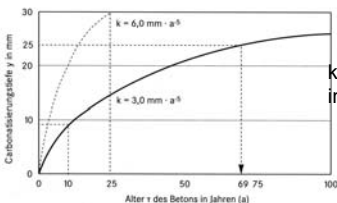
Calculation of carbonation depth – \sqrt{t} -law

Solution of the diffusion equation:

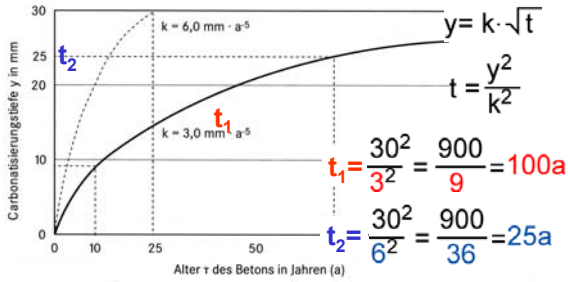
$$y = \sqrt{\frac{2 \cdot D \cdot c_0}{m_0}} \cdot \sqrt{t}$$

$$y = k \cdot \sqrt{t}$$

k = Carbonation coefficient
in mm/a^{0.5}



Calculation of the life span of a carbonating concrete member



Damage mechanism

Sulfidation
Sulfate attack

Sulfidation of concrete

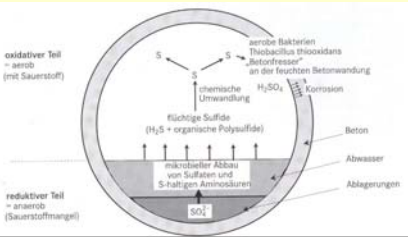
Emissions (e.g. fuel) in the air are oxidised to SO_2 or SO_3 . By reaction with rain „sulphurous acid“ and „sulphuric acid“ is formed

→ Reactions of these acids with cement rock form gypsum
 $\text{Ca}(\text{OH})_2 + \text{H}_2\text{SO}_4 \rightarrow \text{CaSO}_4 \cdot 2 \text{H}_2\text{O}$

But no high penetration depth → surface near effect

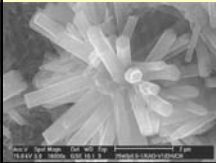
Sulfidation of concrete

In presence of micro organisms gaseous sulfur compounds can also be formed which attack and destroy the concrete.



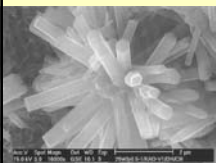
Sulfate attack on concrete

- Important damage mechanism
- Known since 1877 (Michaelis)
 - Structural damage due to the formation of complex compounds ($3 \text{ CaO} \cdot \text{Al}_2\text{O}_3 \cdot 3 \text{ CaSO}_4 \cdot 32 \text{ H}_2\text{O}$)
 - Damage event: 1890 in Magdeburg. Spring water with 2000 mg/l sulfate → 8 cm expansion in 2 years

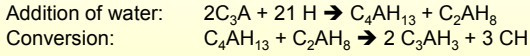


Influence of sulfate – Attack through expansion

- Sulfate attack results in expansion
- Compounds of cement rock react together (**INTERNAL ATTACK**) or together with penetrating chemical agents (**OUTER ATTACK**)

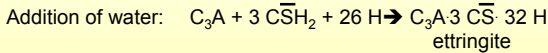


Portland cement – Hydration of C_3A



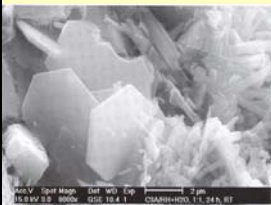
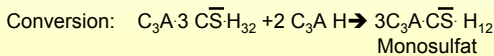
- In absence of sulfate C_3A hydrates in thin plates (calciumaluminate-hydrates)
- **Result:** quick stiffening of the material

Portland cement – Hydration of C_3A in presence of $CaSO_4$ Formation of ettringite



- In presence of gypsum the water-rich compound ettringite is formed
- **needle-structure**
- Result:** Decrease of the reactivity of the C_3A -Phase

Portland cement – Hydration of C_3A in presence of $CaSO_4$ Formation of monosulfate



- The layer get more porous and the reaction starts again.

Types of sulfate attack

Inner sulfate attack:

- Excessive gypsum content in Portland cement
- Cement mortar in contact with gypsum (e.g. reconstruction of brickwork with mortars containing gypsum)

Outer sulfate attack:

- Sulfate containing waters and soils

Inner/Outer attack – Reaction process

Inner sulfate attack:

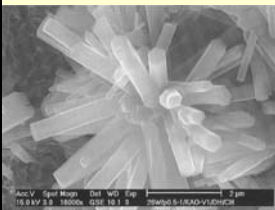
- Comparative quick reaction
- Speed of reaction decreases by time

Outer sulfate attack:

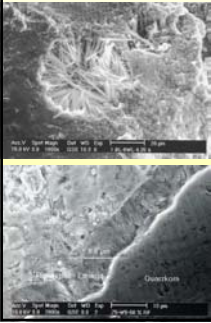
- Time-dependent because of transport process
- High sulfate concentration is necessary
- Speed of reaction increases
- Cation determines speed and amount of reaction too

Properties of sulfate attack

C_3A resp. calciumaluminate hydrates react with gypsum under formation of ettringite
→ 8 times increase in mol volume



Properties of sulfate attack

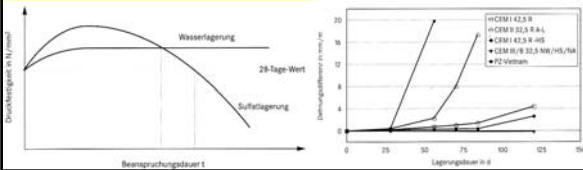


Formation of ettringite in pores and phase boundary layers

Reason:
Large pore area is accessible for precipitation products

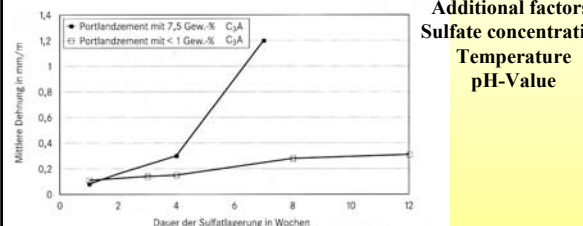
Effect on concrete structure

- Formation of ettringite firstly results in higher strength
- Formation of micro cracks → expansion!!
- Total demolition of the structure



Factors determining the sulfate resistance – C₃A-content

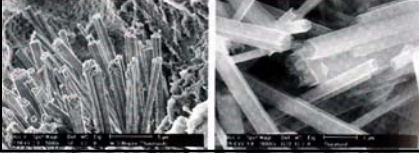
- Sulfate resistance is mainly depending on C₃A-content



Additional factors:
Sulfate concentration
Temperature
pH-Value

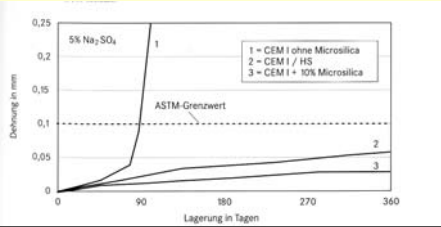
Factors determining the sulfate resistance – Low C_3A -content cements

- Even cements with low C_3A -content can be attacked by sulfate → formation of thaumasite
- Mechanism:
 1. Sulfate exposure to C_3A → Ettringite (actuator)
 2. CO_2/CO_2^- (water) and SiO_2 (CSH-Gel) transform ettringite to THAUMASITE
- Pre-condition: temperature < 10 °C, even better < 5 °C



Factors determining the sulfate resistance – Influence of additives

- Fly ash, sands und micro silica (Puzzolane) improve sulfate resistance → higher density
- This effect is increased when C_3A -content decreases



Type and Concentration of sulfate solution



- $(NH_4)_2SO_4 > MgSO_4 > Na_2SO_4 > CaSO_4$
- E.g. combination of dissolving and expanding attack
- Degree of attack depends on concentration (up to 10 000 mg/l)
 - up to 1000 mg/l: Formation of AFt und AFm
 - up to 1000 mg/l: Formation of gipsium

Grenzwerte für die chemischen Merkmale der Expositionsklassen XA1-XA3 nach DIN EN 206-1

Chemisches Merkmal	Referenzprüfverfahren	XA1	XA2	XA3
Grundwasser				
SO ₄ ²⁻ mg/l in Wasser	EN 196-2	≥ 200 und ≤ 600	> 600 und ≤ 3000	> 3000 und ≤ 6000
pH-Wert	ISO 4316	≤ 4,5 und ≥ 5,5	< 4,5 und ≥ 4,5	< 4,5 und ≥ 4,0
CO ₂ mg/l angreifend	PrEN 13577	≥ 15 und ≤ 40	> 40 und ≤ 100	> 100 bis zur Sättigung
NH ₄ ⁺ mg/l	ISO 7150-1 oder ISO 7150-2	≥ 15 und ≤ 30	> 30 und ≤ 60	> 60 und ≤ 100
Mg ²⁺ mg/l	ISO 7980	≥ 300 und ≤ 1000	> 1000 und ≤ 3000	> 3000 bis zur Sättigung
Boden				
SO ₄ ²⁻ mg/kg insgesamt ¹⁾	EN 196-2 ²⁾	≥ 2000 und ≤ 3000 ³⁾	> 3000 ³⁾ und ≤ 12.000	> 12.000 und ≤ 24.000
Säuregrad in ml/kg	DIN 4030-2	> 200 Baumann-Gully	in der Praxis nicht anzutreffen	

**Critical value,
defined by DIN EN
206-1**

¹⁾ Tabellen mit einer Durchlässigkeit von weniger als 10⁻⁶ m/s dürfen in eine niedrigere Klasse eingestuft werden.
²⁾ Das Prüfverfahren beschreibt die Auslösung von SO₄²⁻ durch Sulfidum; Wissenszulassung darf nicht dessen Anwendung werden, wenn am Ort der Verwendung des Betons Erfahrung hierfür vorzuliegen ist.
³⁾ Falls die Gefahr der Anreicherung von Sulfationen im Beton - zurückzuführen auf wechselladendes Frachten und Durchfrachten oder kapillare Saugen - besteht, ist der Grenzwert von 3000 mg/kg auf 2000 mg/kg zu vermindern.

Influence of concrete quality on sulfate corrosion

- Low W/C-ratio
- high Portland cement content → Increase of sulfate resistance
- Low cement content
- Addition of pozzolanes

Examples for highly endangered buildings



Beispiel: Beton für eine Kläranlage

	Belebungsbecken Sohle, Wände	Nachklärbecken Kronenbereich Laufbahn
Anforderungen	B 35 / Wasserundurchlässigkeit	
	hoher Frostwiderstand; hoher Widerstand gegen starken chemischen Angriff (1000 mg SO ₄ ²⁻ / l im Grund- wasser/Baugrund) ¹⁾	hoher Frost-Tausalz-Wider- stand
Beton	Kiesbeton 0/32 mit Pumpförderung	
	Konsistenz KR	LP-Beton (ε = 5 ± 0,5 Vol.-%) Konsistenz KP
Zementart	CEM III/A 32,5-NW/NA	CEM I 32,5 R
Zementgehalt z	345	370
SFA f	50 ²⁾	-
Wassergehalt w	180	170
w/z	0,52	0,46 < 0,50
w/(z + 0,4f)	0,49 < 0,50	-
Größtkorn	32	32
Zuschlaggehalt g	1728	1710
Körnung bei	M.-% 0/2a = 34 2/8 = 18 8/16 = 24 16/32 = 24	
Sieblinie im Bereich AB		
Zusatzmittel	BV	BV, LP
Druckfestigkeit d = 28 / 56 Tage	N/mm ² 42,8 / 50,2 ²⁾	43,0 / -
Wassereindring- tiefe e _{ys}	mm 22 < 30	20 < 30
CDF-Test	-	Bestanden

¹⁾ Nach RILJ DAISib 09/96 „Verwendung von Flugasche nach DIN EN 450 im Betonbau“ gilt:
f / (z + f) ≥ 0,10 bei Einsatz von CEM III/A

²⁾ Vereinbartes Nachweissalter beim Beton mit CEM III/A ist d = 56 Tage

FH Karlsruhe

UNIVERSITY OF APPLIED SCIENCES
HOCHSCHULE FÜR TECHNIK

Case study:
concrete for
waste water
treatment plant

Waste water treat-
ment plant



Forschungszentrum Karlsruhe
in der Helmholtz-Gemeinschaft

FH Karlsruhe
UNIVERSITY OF APPLIED SCIENCES
HOCHSCHULE FÜR TECHNIK

Cement based materials in permanent contact with water

Forschungszentrum Karlsruhe
in der Helmholtz-Gemeinschaft

FH Karlsruhe
UNIVERSITY OF APPLIED SCIENCES
HOCHSCHULE FÜR TECHNIK

Why are drinking water reservoirs coated?

- Hygienic storing
- Repairing of entrapments (e.g. cracks)
- Abrasion layer against chemical and mechanical influences
- Easier cleaning
- appearance

Corrosion of mineral materials

Characterisation of the damage

- Appearance
- Mechanical properties
- Chemical composition
- Kinetics



Damage characterisation

Appearance

- Circular damage spots (diameter up to 5 cm)
- Often brown discoloration
- Often vertical, horizontal or orthogonal archetypes



Damage characterisation

Mechanical properties

- Defect material is softened
- Material can be easily removed, e.g. by cleaning



Damage characterisation

Chemical composition

- Ca(OH)₂-content equals zero
- Increased CaCO₃-content



Damage characterisation

Kinetic

- High speed and local increase in corrosion
- Damage appears already six month after application



Possible causes for that damage

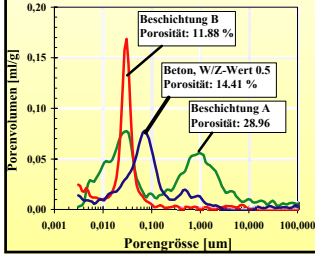
- Water attacks chemically
- Microorganism
- Acidic cleaners
- Hydrolysis of the cement based material



Mechanism of hydrolysis

1. Reaction step: ion-transport

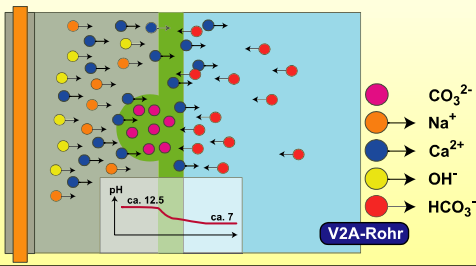
- intake of HCO_3^- -ions
- export of Na^+ , K^+ and Ca^{2+} -ions



Mechanism

- diffusion controlled
- transport resistance is structure dependent (total porosity, content capillary pores)
- accelerated by electric fields

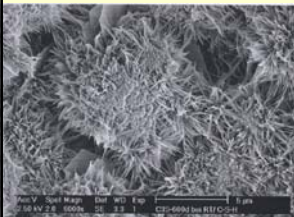
Schematic demonstration: Hydrolysis of cement based materials



Hydrolysis mechanism

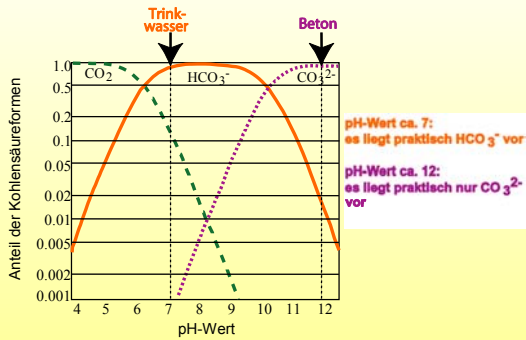
2. Reaction step: chemical reaction

- pH-dependent transformation of HCO_3^- ions in CO_3^{2-} ions
- Precipitation of CaCO_3 by consuming Ca(OH)_2
- degradation CSH-phases to SiO_2 , Al_2O_3 and CaCO_3



Hydrolysis is a coupled process of transport and chemical reaction

„Lime-carbonic acid“-balance



Concept for quality insurance „Coatings for drinking water reservoirs“ in Switzerland

Tests:

1. Step: withdrawal of drilling core (Ø 50 mm)
2. Step : Determination of **layer thickness**
3. Step : Measurement of **total porosity** and **pore size distribution** (Hg-porosimetry)
4. Step: Determination of **calcium carbonate-** and **calcium hydroxide-**content

Concept for quality insurance „Coatings for drinking water reservoirs“ in Switzerland

Demands:

- Layer thickness > 10 mm
Total porosity: < 15% (p=2500 bar)
1. Maximum in pore size distribution: < 0.1 µm
Chemical analysis: -
