



**Near ground surface, engineered structures become unstable in response to extreme changes in moisture flux boundary conditions**



**Unsaturated Soil Technology**

**CHAPTER 10 MEASUREMENT OF SHEAR STRENGTH  
PARAMETERS** .....

**10.1- SPECIAL DESIGN CONSIDERATIONS** .....

- 10.1.1 Axis-Translation Technique .....
- 10.1.2 Pore-Water Pressure Control or Measurement .....
- Saturation procedure for a high air entry disc .....
- 10.1.3 Pressure Response Below the Ceramic Disc .....
- 10.1.4 Pore-Air Pressure Control or Measurement .....
- 10.1.5 Water Volume Change Measurement .....
- 10.1.6 Air Volume Change Measurement .....
- 10.1.7 Overall Volume Change Measurement .....
- 10.1.8 Specimen Preparation .....
- 10.1.9 Backpressuring to Produce Saturation .....

**10.2 TEST PROCEDURES FOR TRIAXIAL TESTS** .....

- 10.2.1 Consolidated Drained Test .....
- 10.2.2 Constant Water Content Test .....
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**10.3 TEST PROCEDURES FOR DIRECT SHEAR TESTS** .....

**10.4 TYPICAL TEST RESULTS** .....

- 10.4.1 Triaxial Test Results .....
- Consolidated drained triaxial tests .....
- Constant water content triaxial tests .....
- Nonlinear shear strength versus matric suction .....
- Undrained and unconfined compression tests .....
- 10.4.2 Direct Shear Test Results .....

**Design features**

**Test procedures**

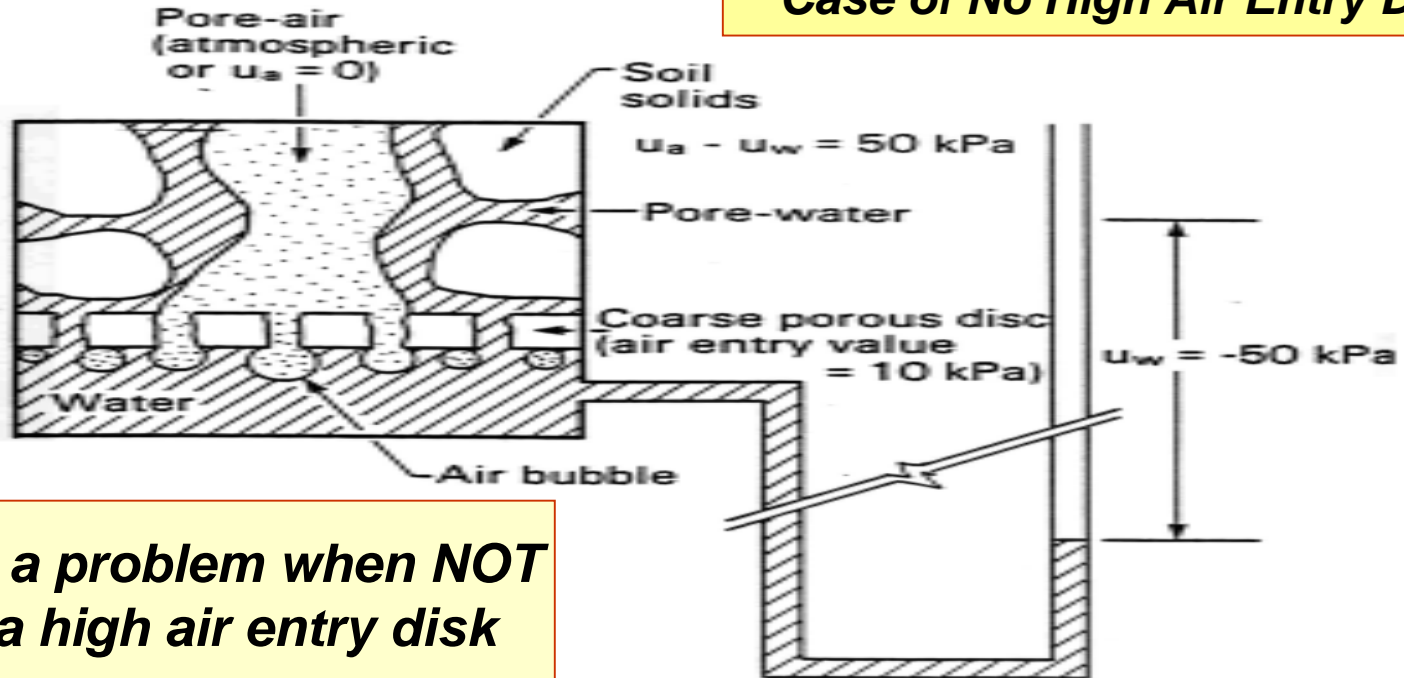
**Typical results**



CHAPTER 10  
MEASUREMENT OF SHEAR STRENGTH  
PARAMETERS

Axis-Translation Technique

Case of No High Air Entry Disk



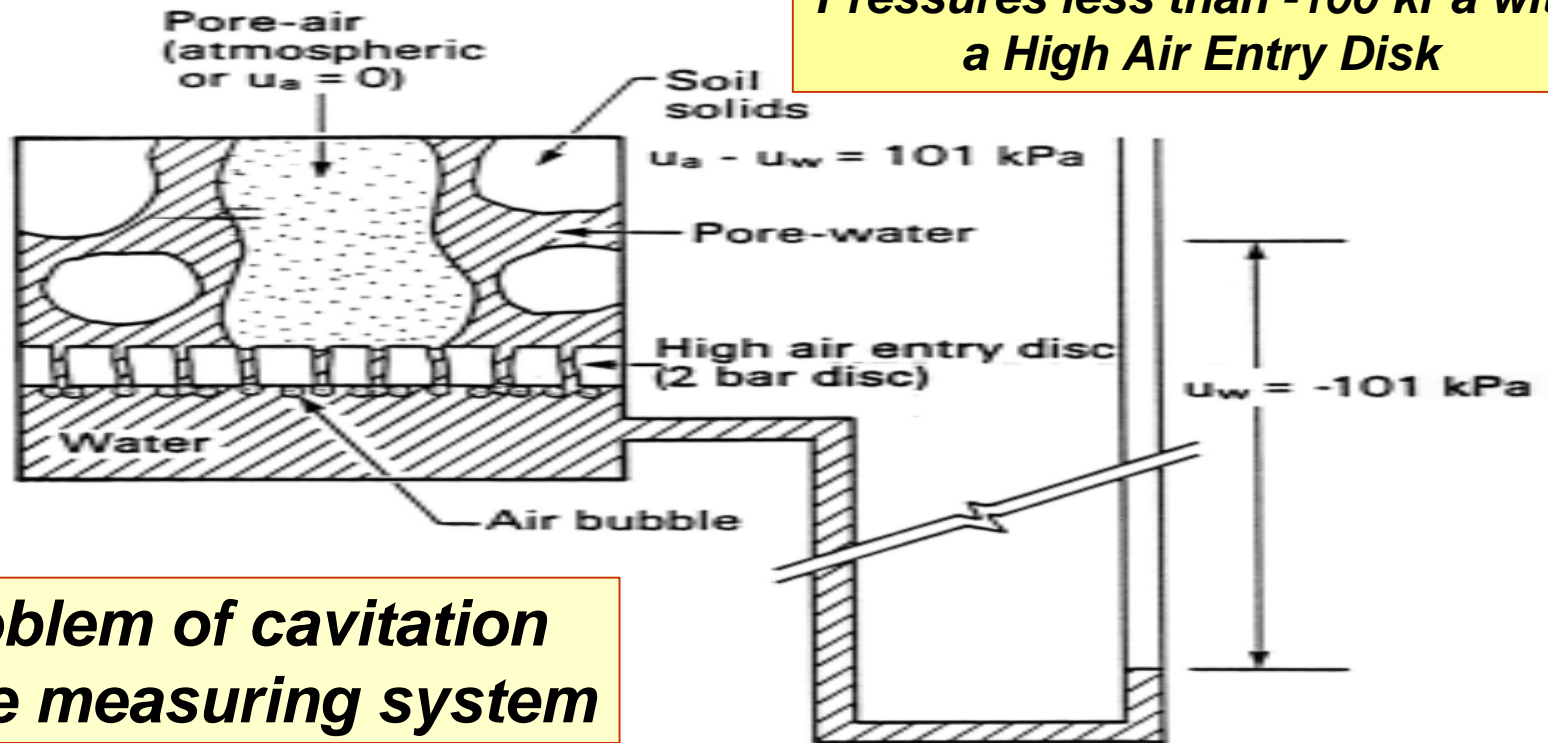
There is a problem when NOT using a high air entry disk

(a) Air intrusion through porous disk when its air entry value is exceeded

Direct measurement of pore-water pressure in an unsaturated soil specimen (not to scale)

## Axis-Translation Technique

**Case of Negative Pore-water Pressures less than -100 kPa with a High Air Entry Disk**



**Problem of cavitation  
In the measuring system**

(b) Water cavitation in the measuring system

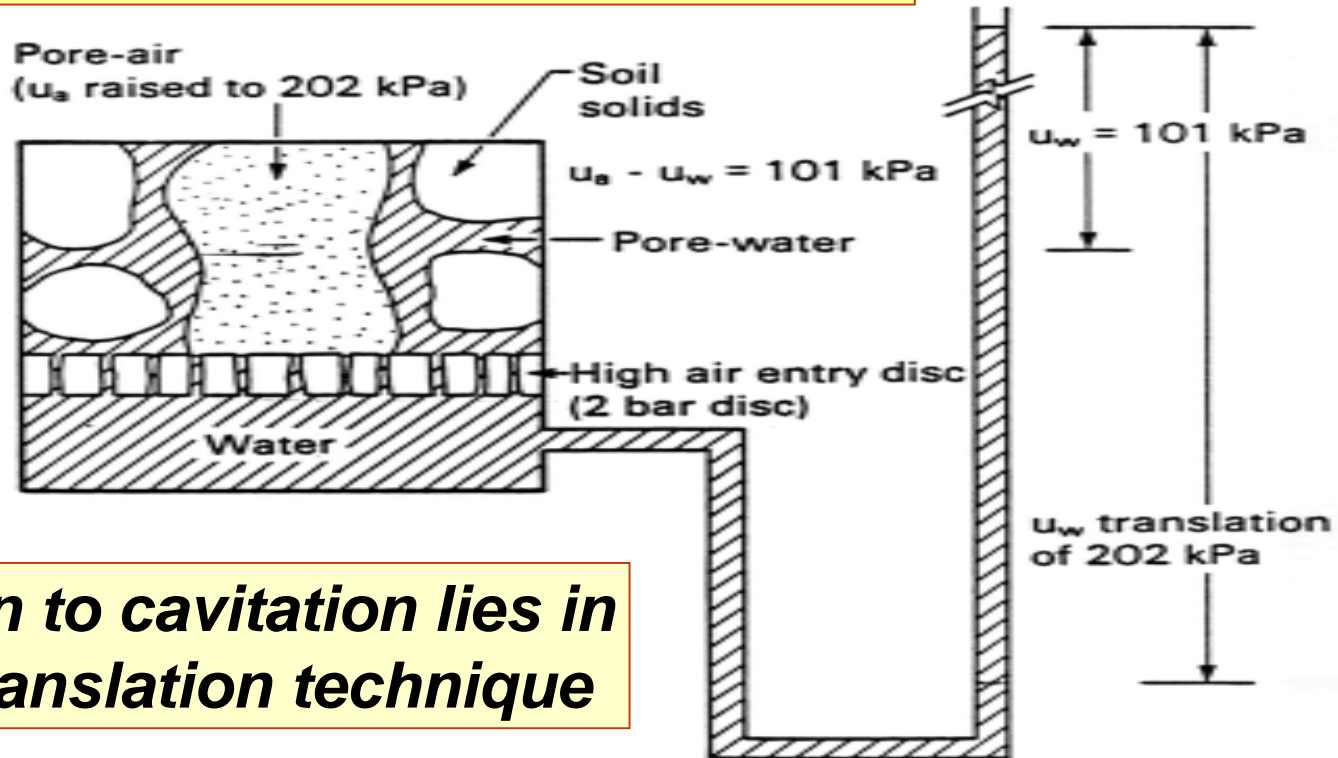
Direct measurement of pore-water pressure in an unsaturated soil specimen (not to scale)



**Unsaturated Soil Technology**

## Axis-Translation Technique

### Case of High Air Entry Disk and Axis-translation



**Solution to cavitation lies in Axis-translation technique**

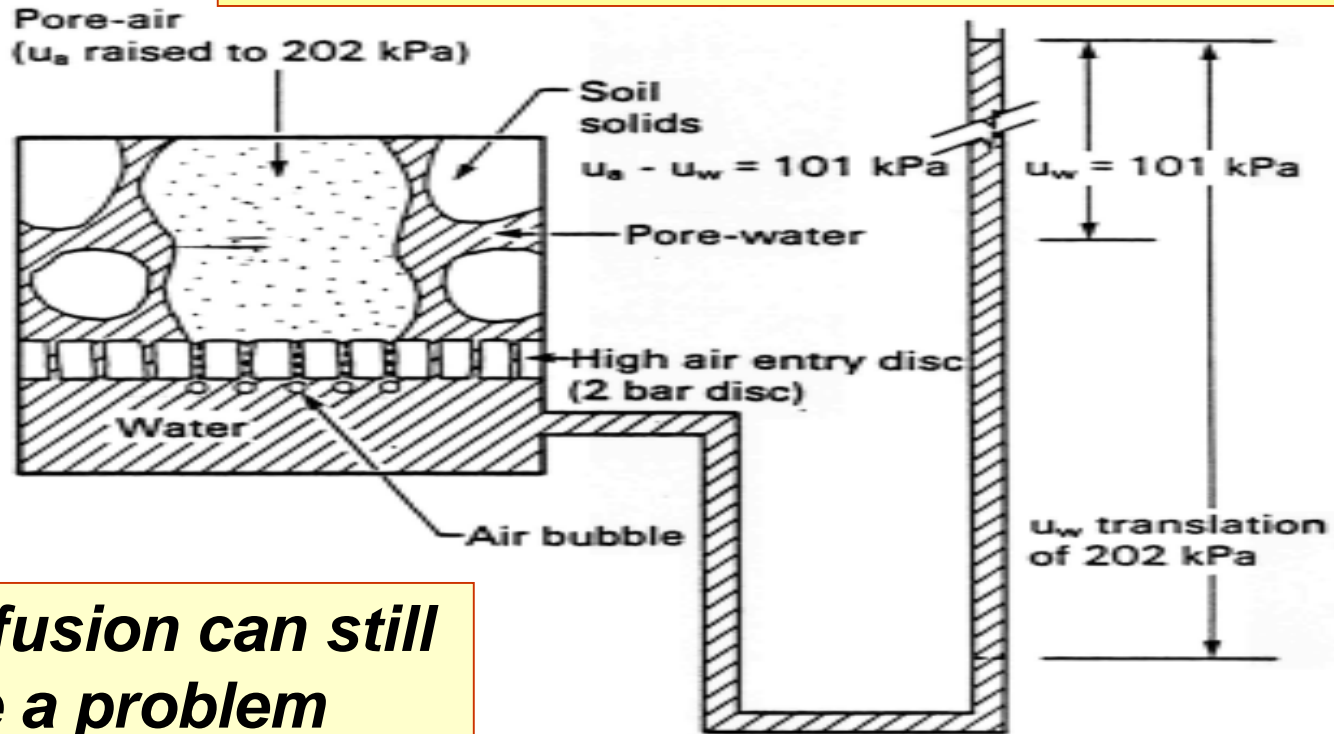
(a) Axis-translation of 101 kPa

Measurements of pore-water pressure in an unsaturated soil specimen using the axis-translation technique (not to scale)



## Axis-Translation Technique

### Case of High Air Entry Disk and Axis-translation

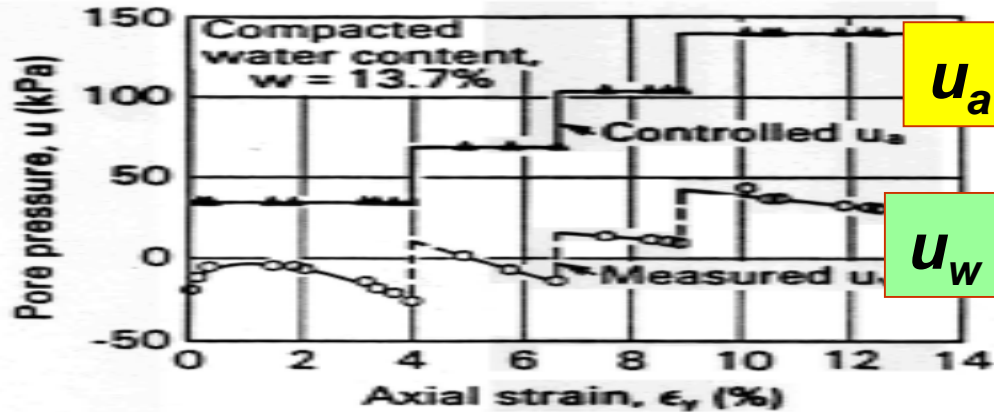


***Air diffusion can still be a problem***

(b) Air diffusion through the high air entry disk

Measurements of pore-water pressure in an unsaturated soil specimen using the axis-translation technique (not to scale)

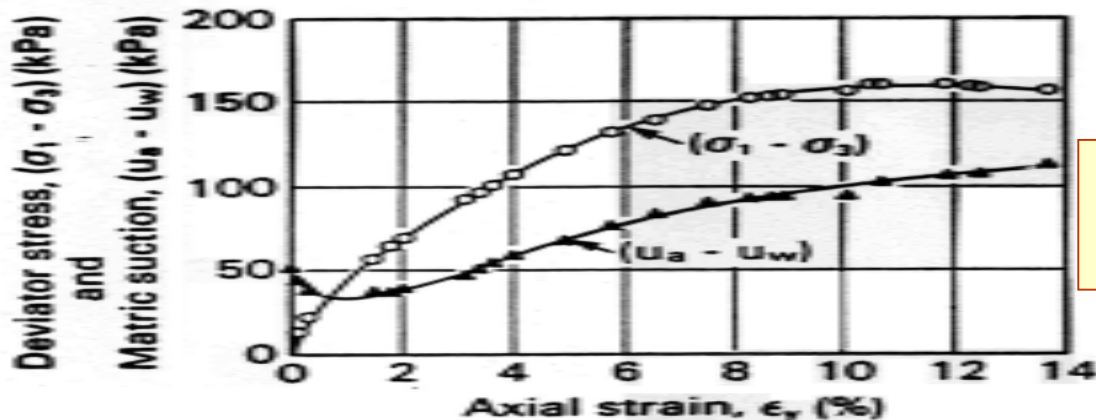
**Does axis-  
Translation  
really work?**



**$u_a$  controlled**

**$u_w$  measured**

(a) Pore pressures versus axial strain curve



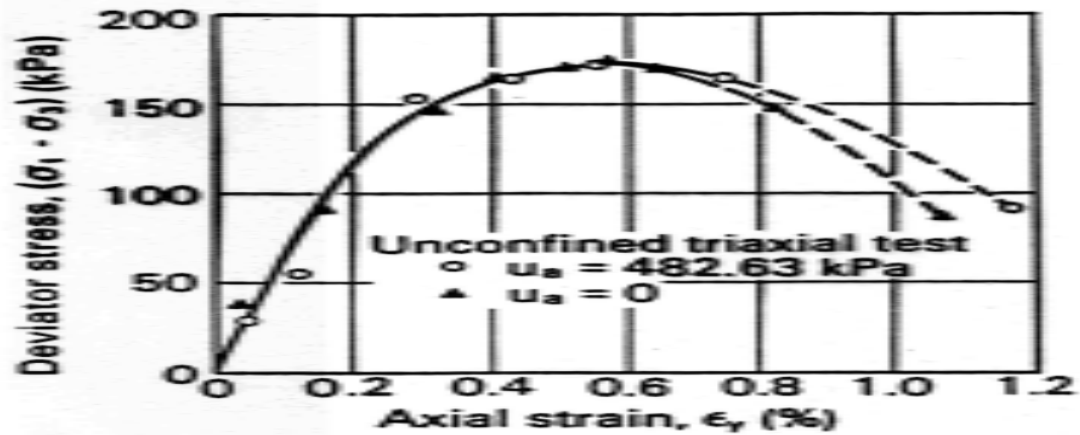
**Suction  
= 110 kPa**

(b) Deviator stress and matric suction versus axial strain curves

"Unconfined triaxial" test on a compacted Selset clay specimen at various pore-air pressures (from Bishop and Blight, 1963)

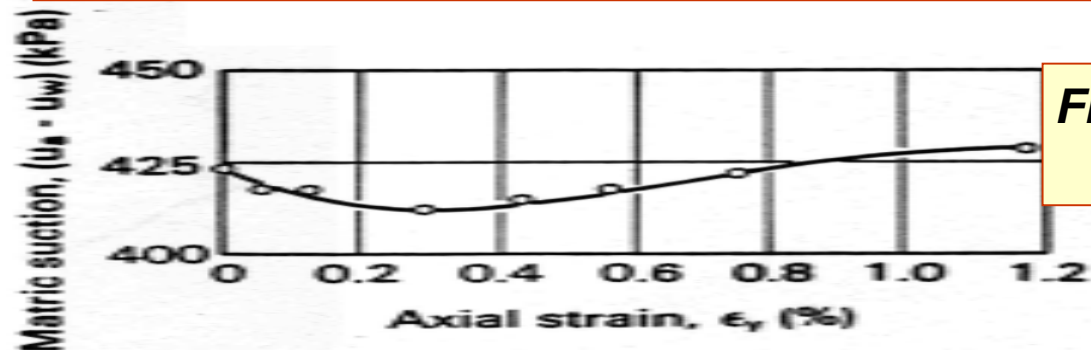


**Does axis-Translation really work?**



(a) Deviator stress versus axial strain curve

$u_a = 483$  kPa;  $u_w = 69$  kPa (Suction = 414 kPa)



**Final Suction = 430 kPa**

(b) Matric suction versus axial strain during the test with axis-translation

"Unconfined triaxial" tests on a compacted Talybont clay with and without axis-translation (from Bishop and Blight, 1963)



### High Air Entry Disks at Imperial College (From Blight, 1961)

Type	Porosity, $n$ (%)	Coefficient of Permeability, $k_d$ (m/s)	Air Entry Value, $(u_a - u_w)$ (kPa)
Doulton Grade P6A	23	$2.1 \times 10^{-9}$	152
Aerox "Celloton" Grade VI	46	$2.9 \times 10^{-8}$	214
kaolin-consolidated from a slurry and fired	45	$6.2 \times 10^{-10}$	317
Kaolin-dust pressed and fired	39	$4.5 \times 10^{-10}$	524

### High Air Entry Disks Manufactured by Soilmoisture Equipment Corporation

Type	Approximate Pore Diameter ( $\times 10^{-3}$ mm)	Coefficient of Permeability, $k_d$ (m/s)	Air Entry Value Range, $(u_a - u_w)$ (kPa)
1/2 bar (high flow)	6.0	$3.11 \times 10^{-7}$	48-62
1 bar	2.1	$3.46 \times 10^{-9}$	138-207
1 bar (high flow)	2.5	$8.6 \times 10^{-8}$	131-193
2 bar	1.2	$1.73 \times 10^{-9}$	241-310
3 bar	0.8	$1.73 \times 10^{-9}$	317-483
5 bar	0.5	$1.21 \times 10^{-9}$	> 550
15 bar	0.16	$2.59 \times 10^{-11}$	> 1520

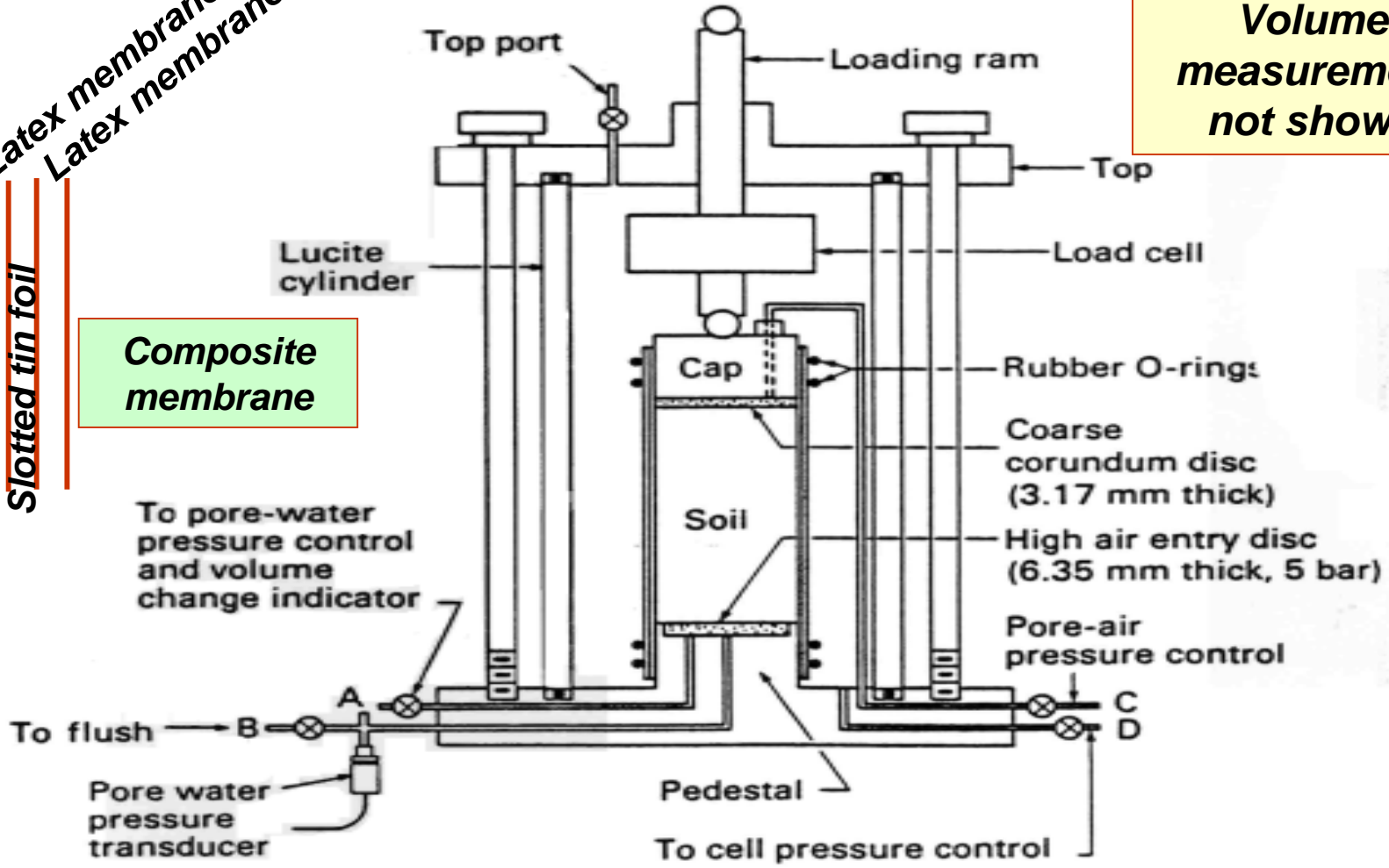


Latex membrane  
Latex membrane

Slotted tin foil

Composite membrane

Volume measurement not shown

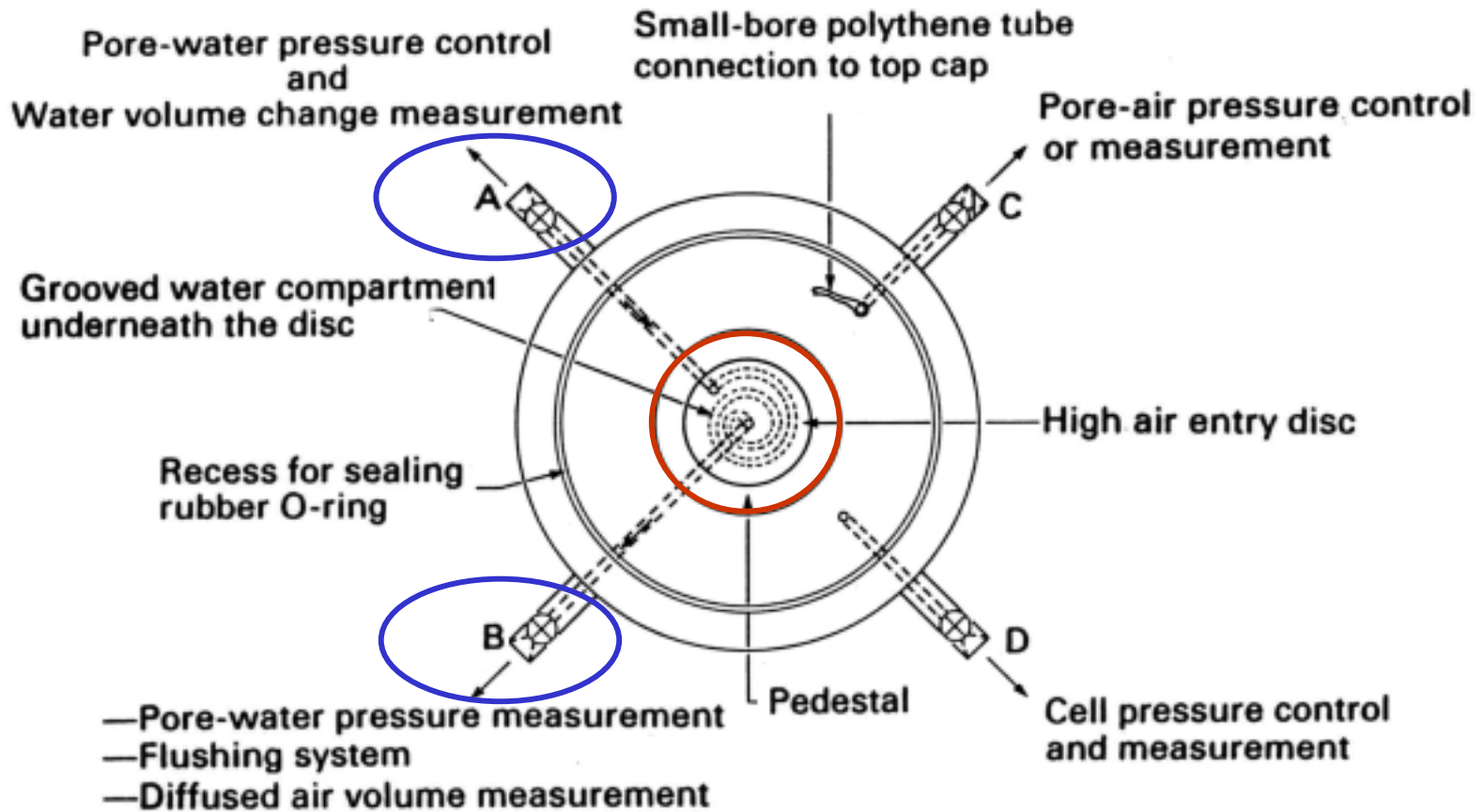


Primary modification is the high air entry disk sealed into the base pedestal

Modified triaxial cell for testing unsaturated soils



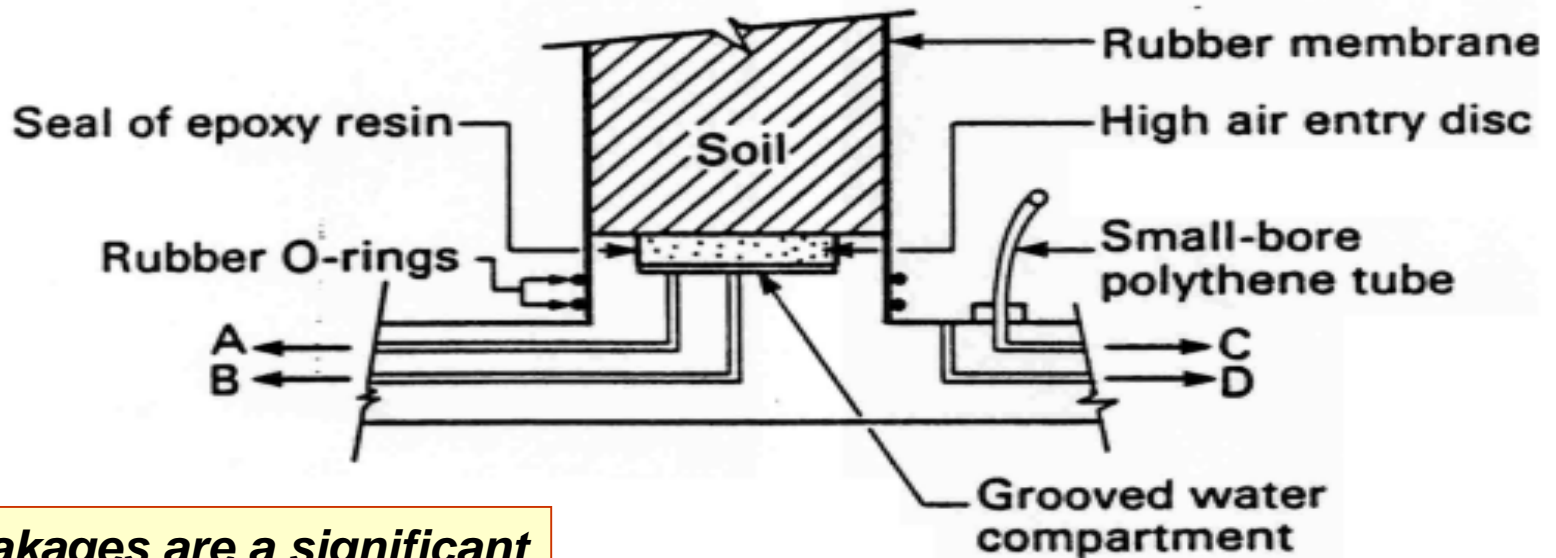
**Unsaturated Soil Technology**



(a) Plan view of the base plate with its outlet ports

Triaxial base plate for unsaturated soil testing





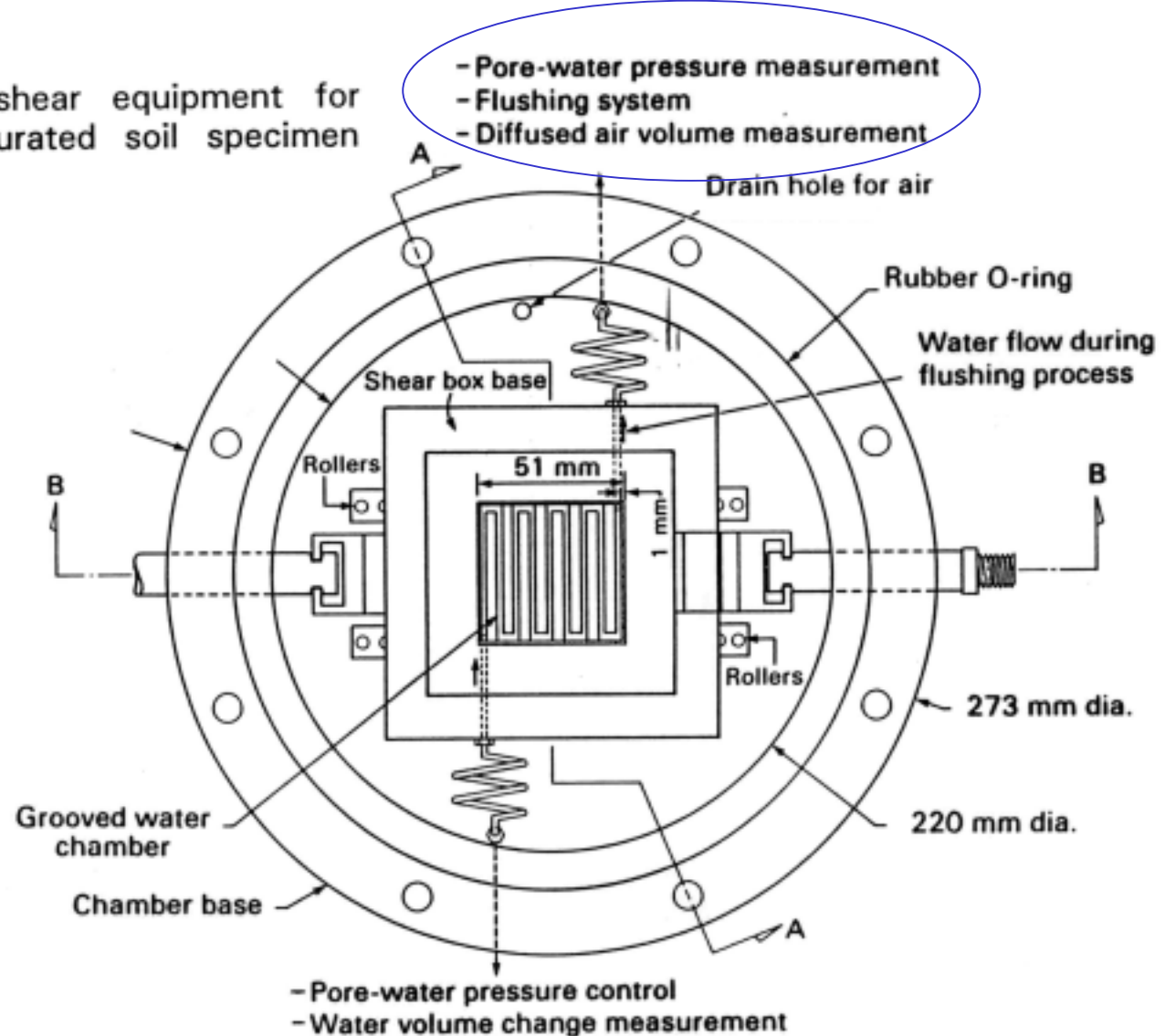
***Leakages are a significant problem in unsaturated soil testing***

(b) Cross-section of a base plate with a high air entry disk

Triaxial base plate for unsaturated soil testing

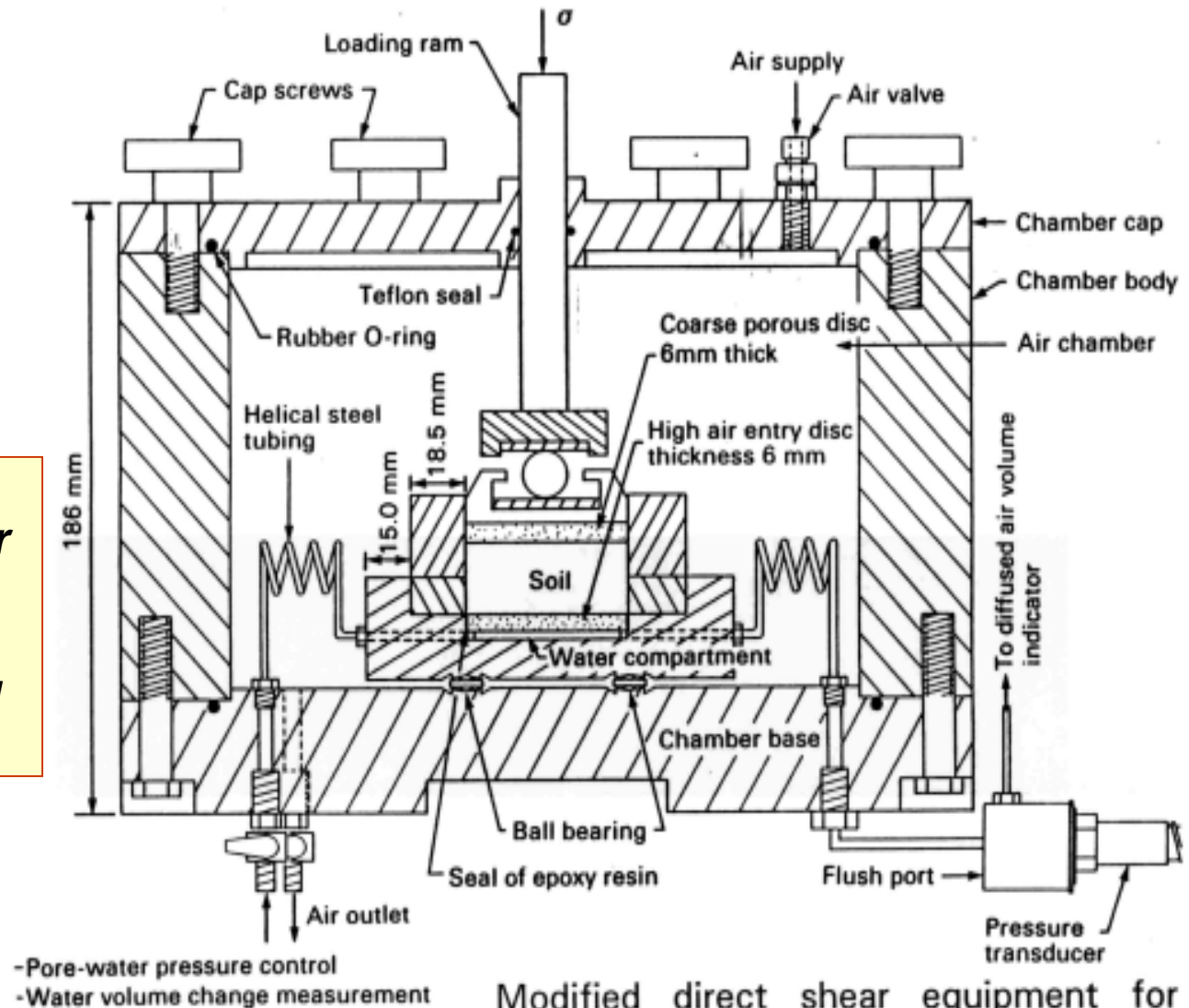


Modified direct shear equipment for testing an unsaturated soil specimen (from Gan, 1986)



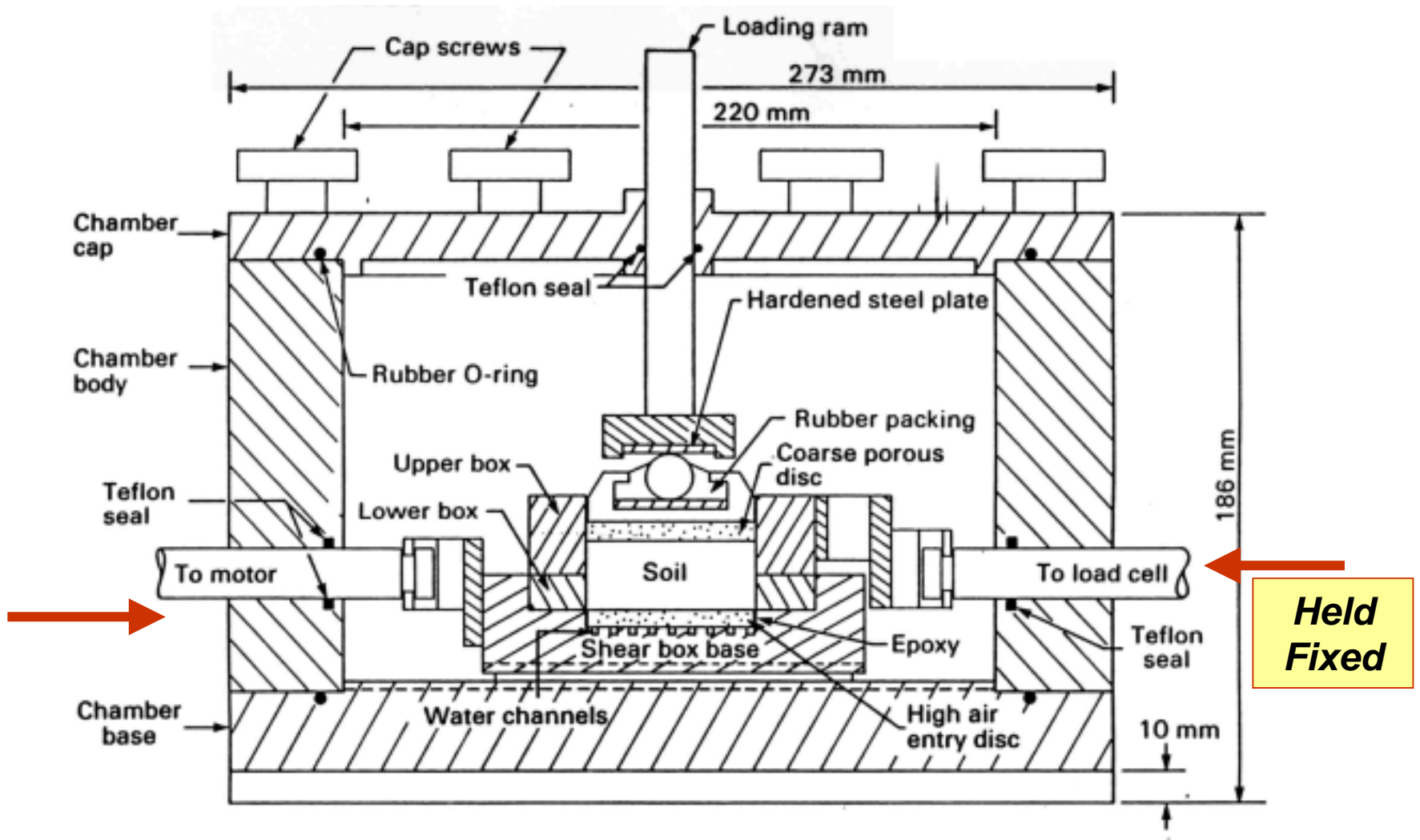
(a) Plan view of the pressure chamber of a direct shear box

**Bottom part of shear box must move because of the vertical piston load**



(b) Cross-sectional view A-A of a direct shear box and pressure chamber

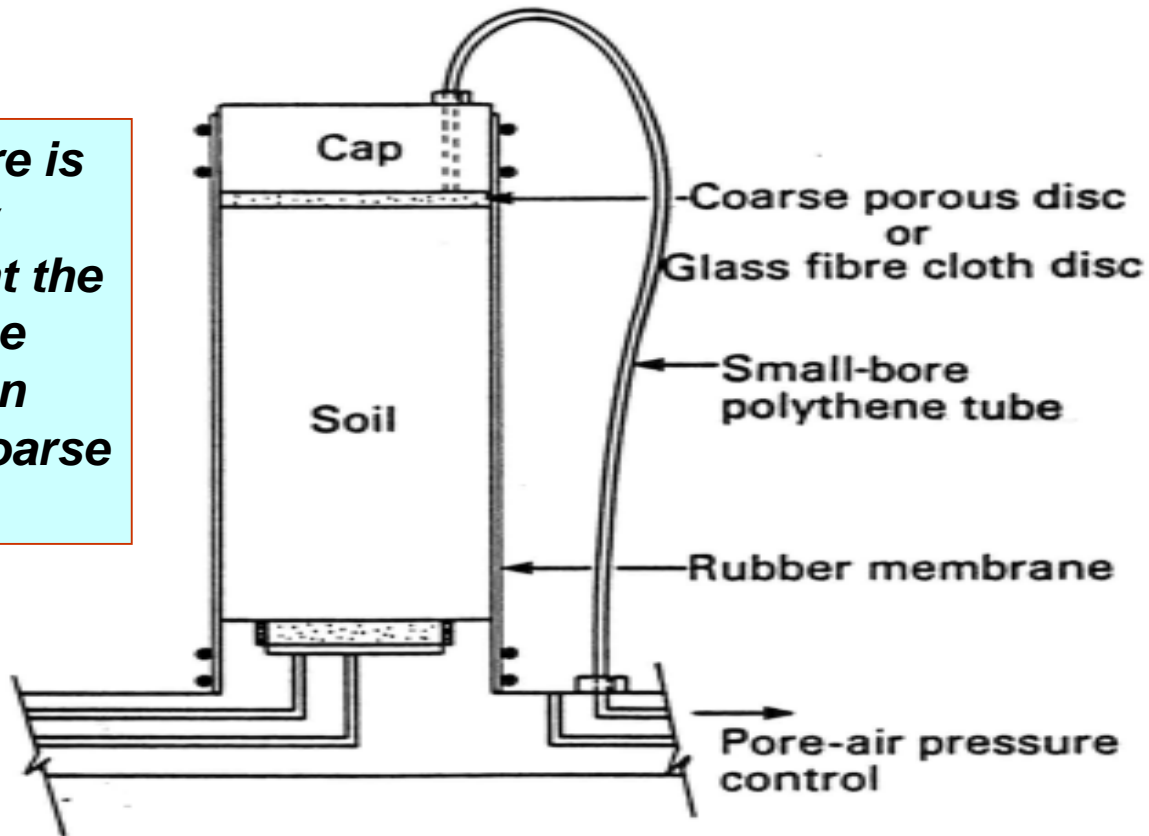
Modified direct shear equipment for testing an unsaturated soil specimen (from Gan, 1986)



Modified direct shear apparatus for testing unsaturated soils (from Gan and Fredlund, 1988)



*Air pressure is usually controlled at the top of the specimen through a coarse disk*



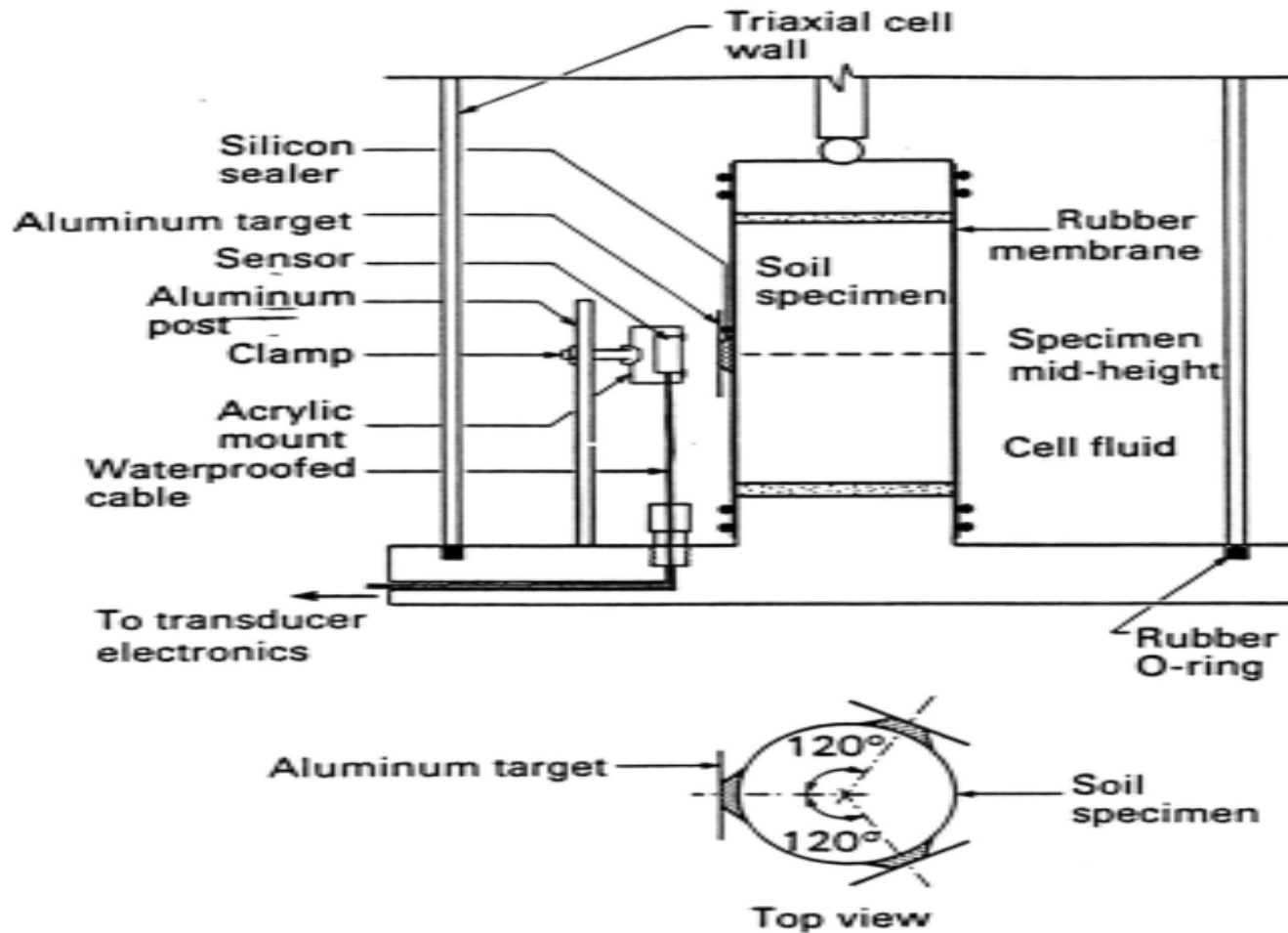
Pore-air pressure control system



**Unsaturated Soil Technology**

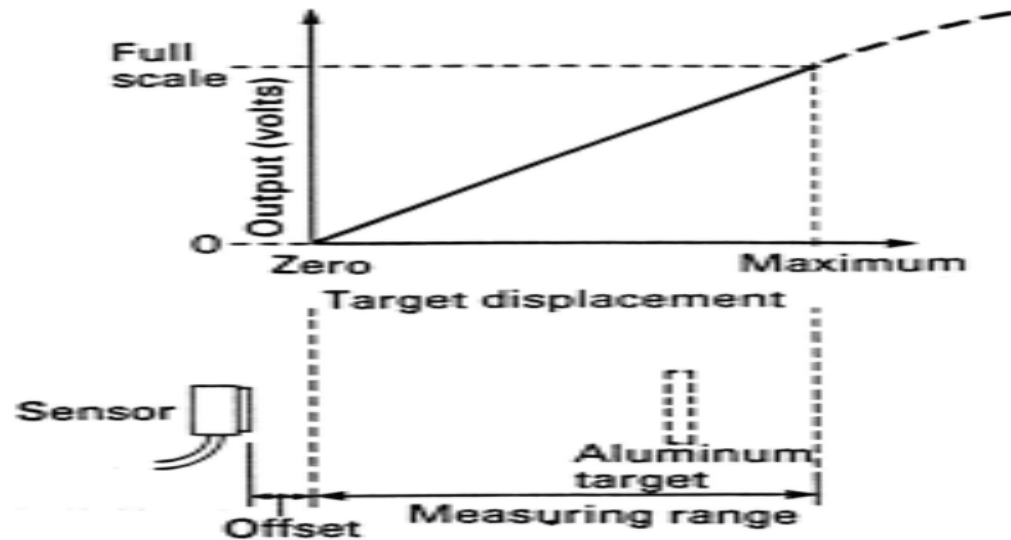


# Measurement of overall volume change

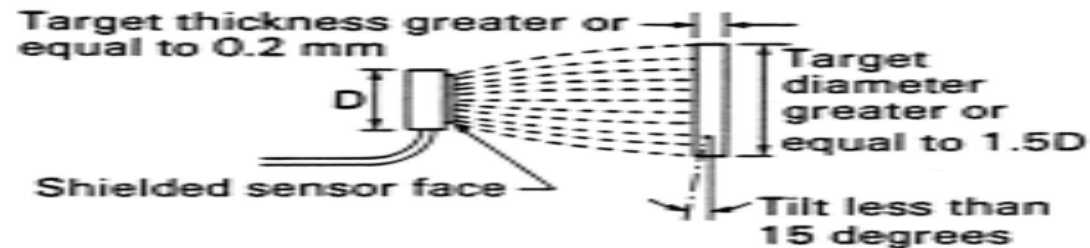


Installation of a non-contacting radial deformation transducer (from Drumright, 1987)

**Require three non-contact sensors but still not a perfect solution for overall volume change**



(a) Calibration

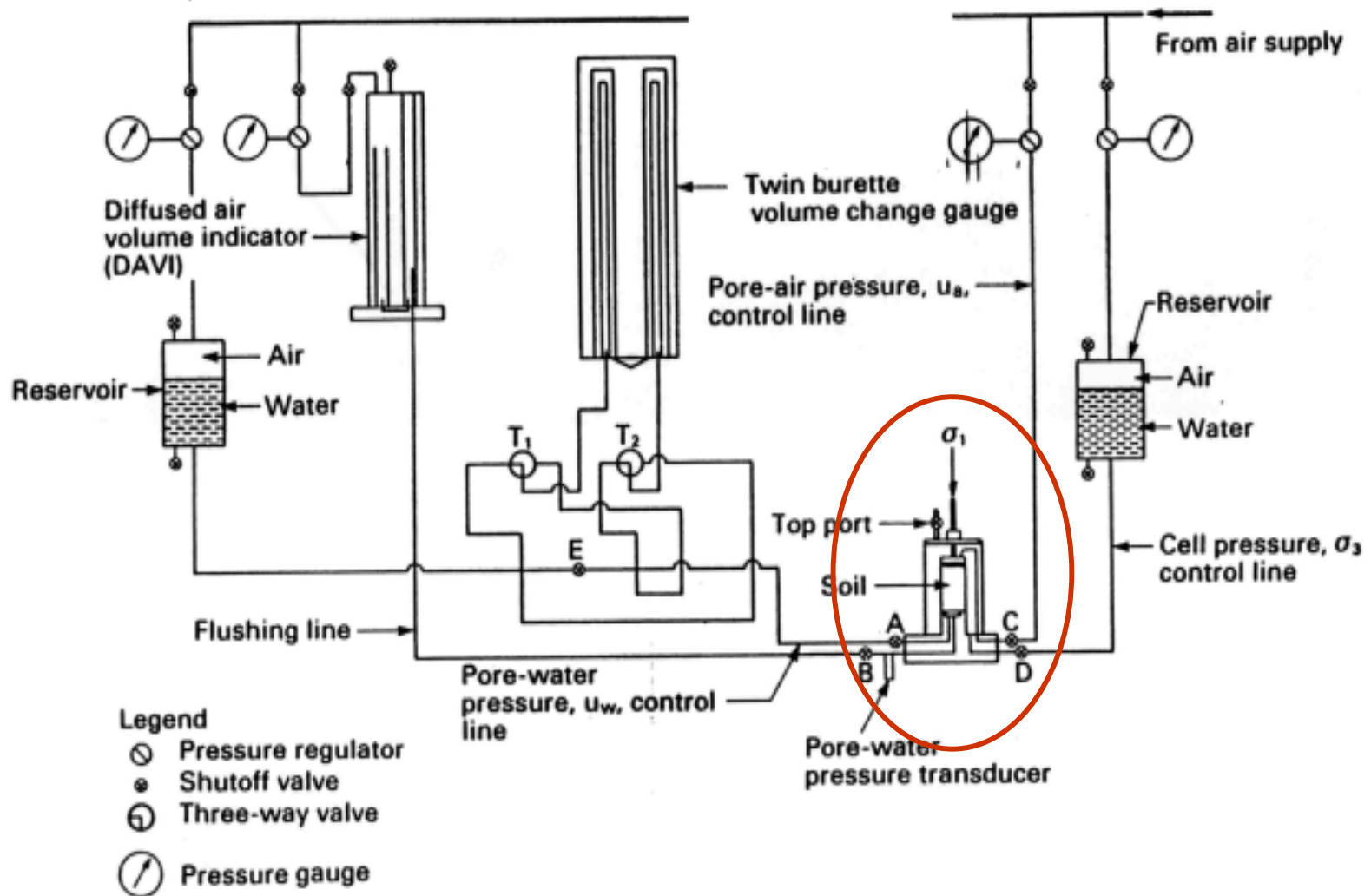


(b) Installation requirements

Non-contacting radial deformation transducer, KD-2310 series, model 4SB, shielded, button-type (from Karman Science Corporation)

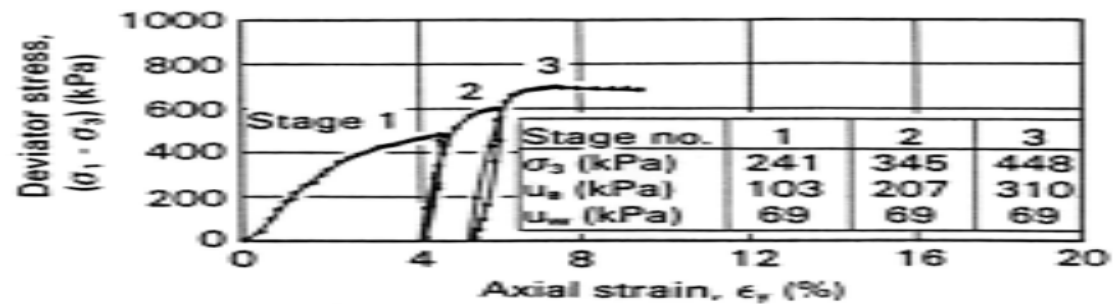


**Unsaturated Soil Technology**

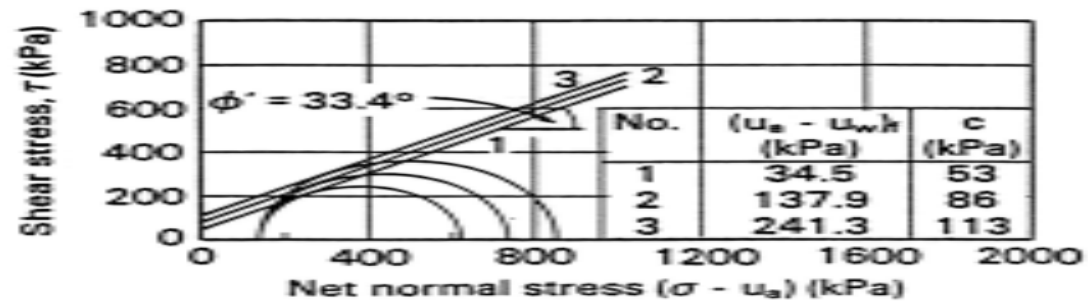


Schematic diagram of the control board and plumbing layout for the modified triaxial apparatus

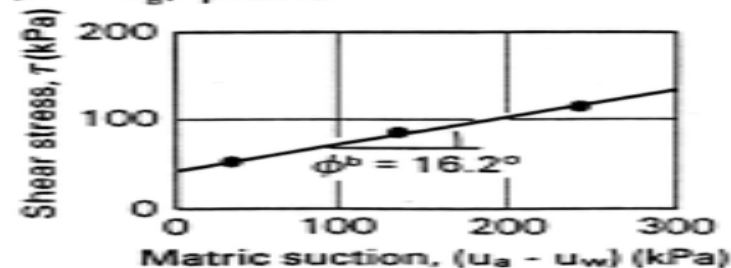
**Typical  
Multistage  
Triaxial Test  
Results on  
Decomposed  
Granite**



(a) Deviator stress versus strain curve



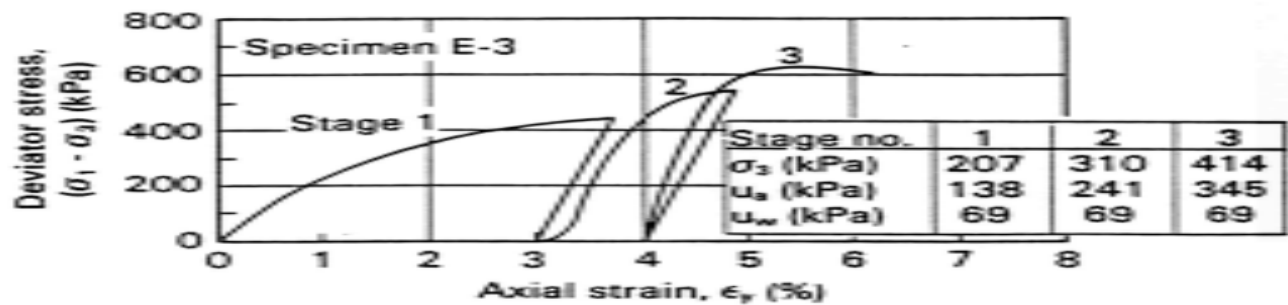
(b) Failure envelope projection onto the  $\tau$  versus  $(\sigma - u_a)$  plane



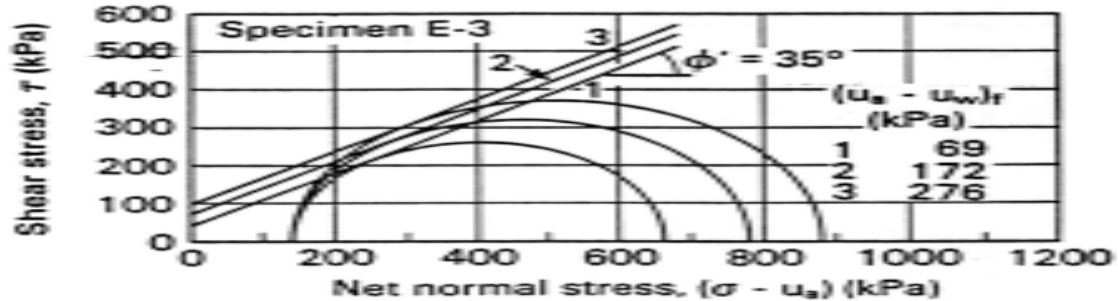
(c) Intersection line between the failure envelope and the  $\tau$  versus  $(u_a - u_w)$  plane

Stress versus strain curves and two-dimensional presentations of the failure envelope for decomposed granite specimen no. 10 (from Ho and Fredlund 1982a)

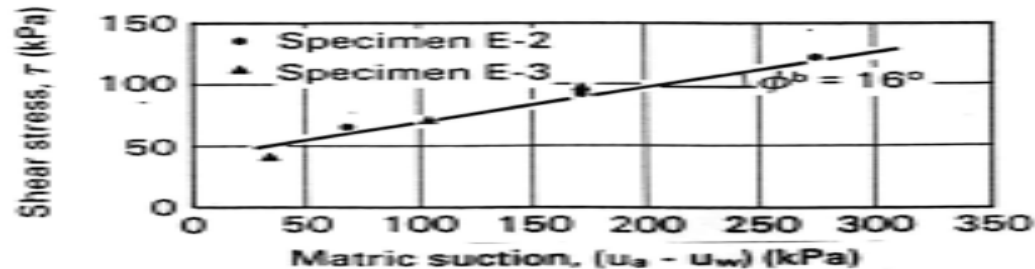
**Typical  
Multistage  
Triaxial Test  
Results on  
Silt  
Specimen  
E-3**



(a) Deviator stress versus strain curve



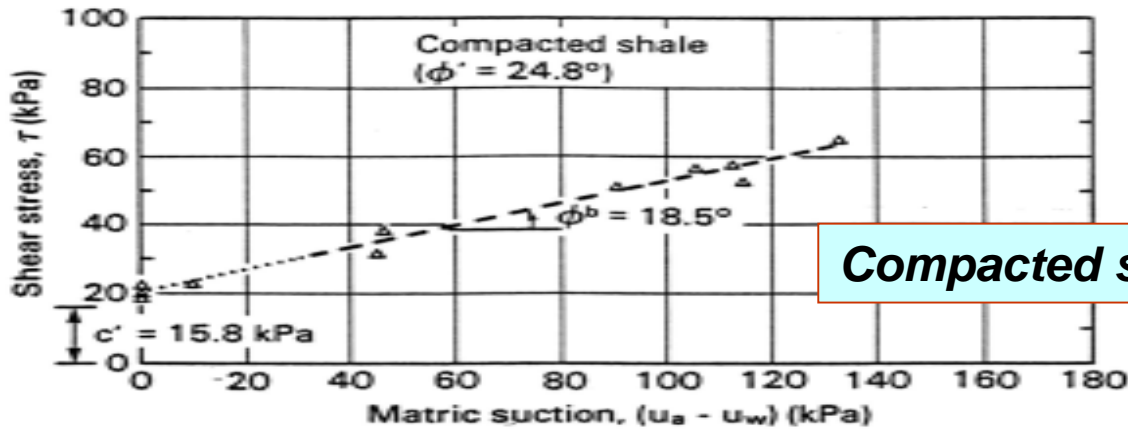
(b) Failure envelope projection onto the  $\tau$  versus  $(\sigma - u_a)$  plane



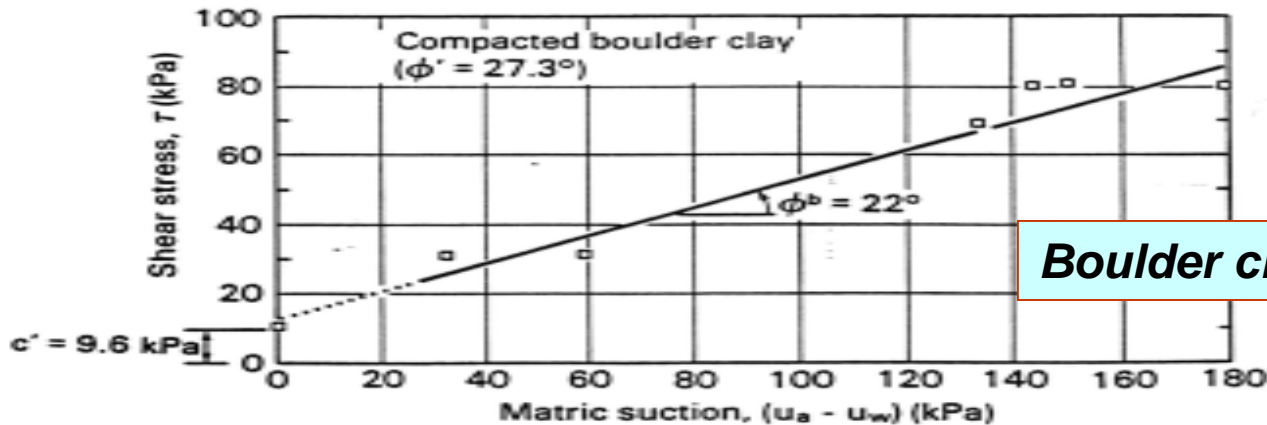
(c) Intersection line between the failure envelope and the  $\tau$  versus  $(u_a - u_w)$  plane

Stress versus strain curves and two-dimensional presentations of the failure envelope for Tappen-Notch Hill Silt specimen no. E-3 (from Krahn, Fredlund and Klassen, 1987)

**Increase in strength with matric suction**

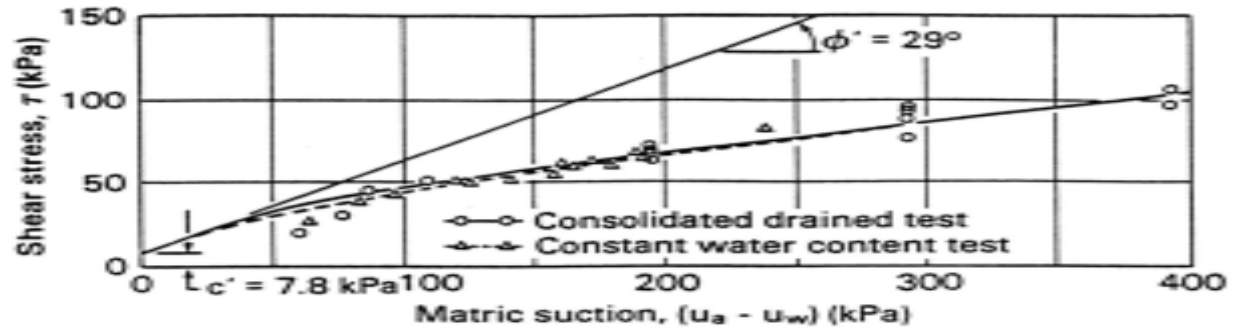


Intersection line between the failure envelope and the  $\tau$  versus  $(u_a - u_w)$  plane for a compacted shale (data from Bishop, Alpan, Blight and Donald, 1960)

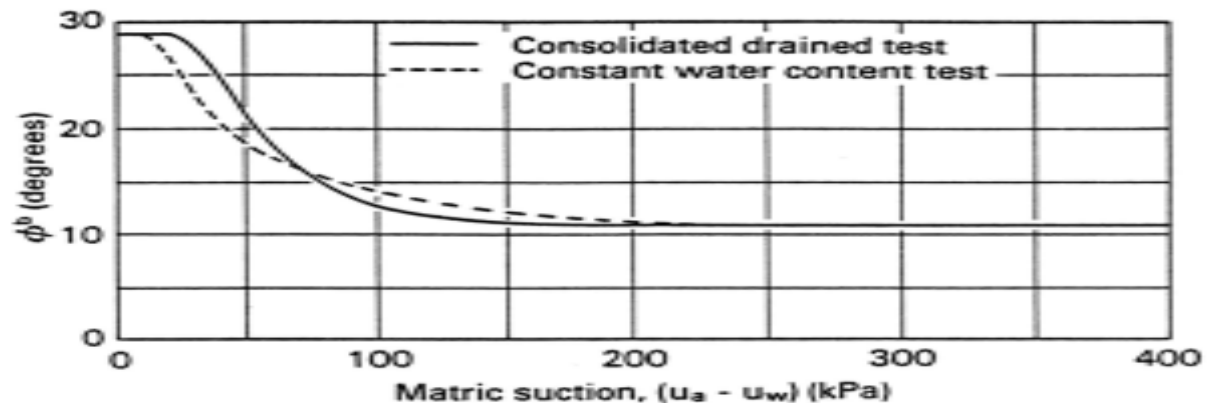


Intersection line between the failure envelope and the  $\tau$  versus  $(u_a - u_w)$  plane for a compacted boulder clay (data from Bishop, Alpan, Blight and Donald, 1960)

**Increase in strength with matric suction for Dhanauri clay compacted at a low density**



(a) Curved failure envelopes for Dhanauri clay compacted to a low density

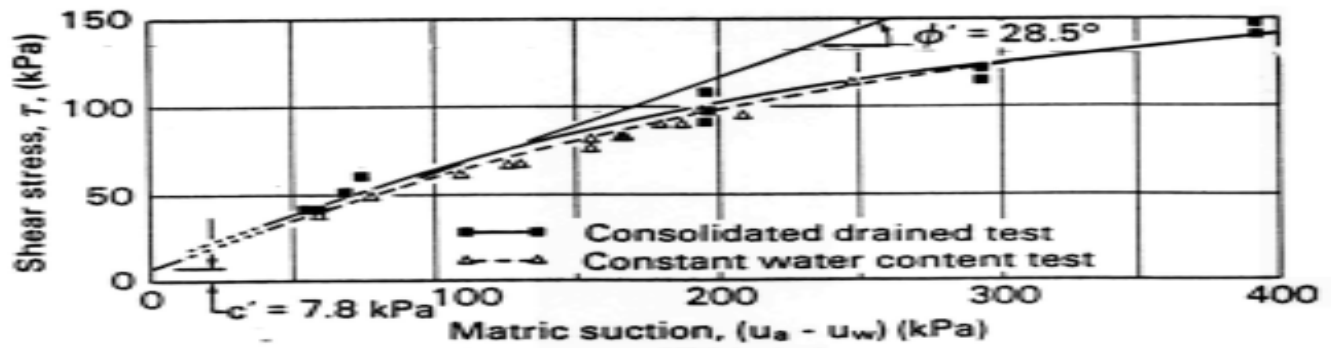


(b) Nonlinear relationship between  $\phi^b$  and matric suction

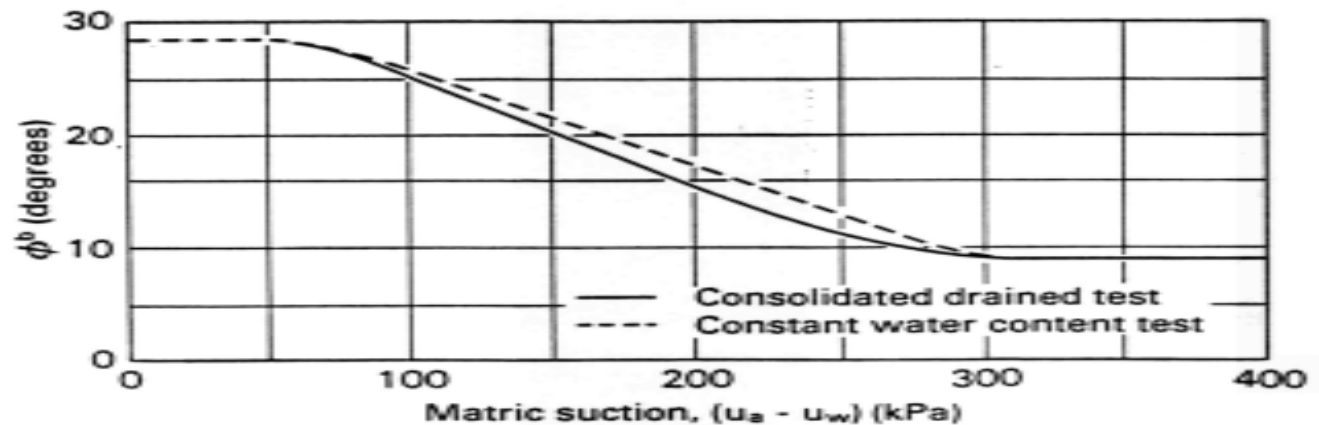
Nonlinearity in the failure envelope with respect to matric suction for Dhanauri clay compacted to a low density



**Increase in strength with matric suction for Dhanauri clay compacted at a high density**



(a) Curved failure envelopes for Dhanauri clay compacted to a high density (data from Satija, 1978)



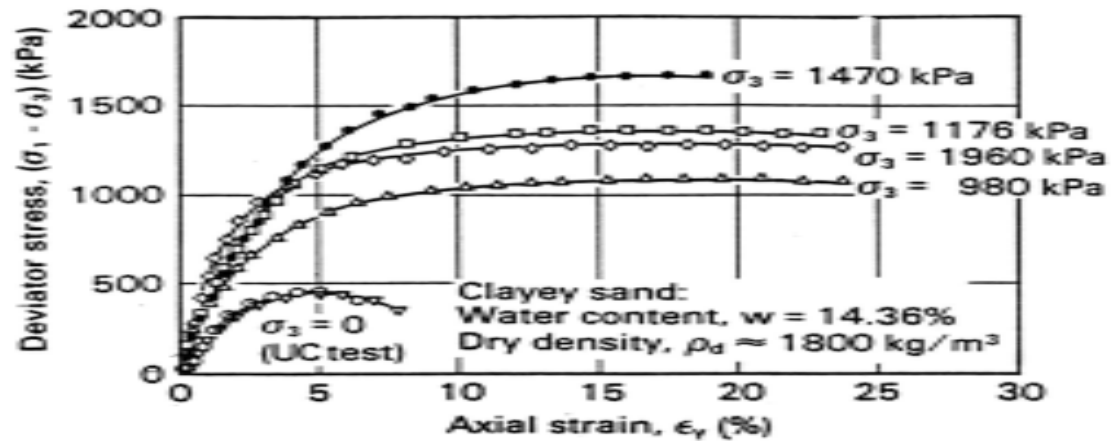
(b) Nonlinear relationship between  $\phi^b$  and matric suction

Nonlinearity in the failure envelope with respect to matric suction for Dhanauri clay compacted to a high density

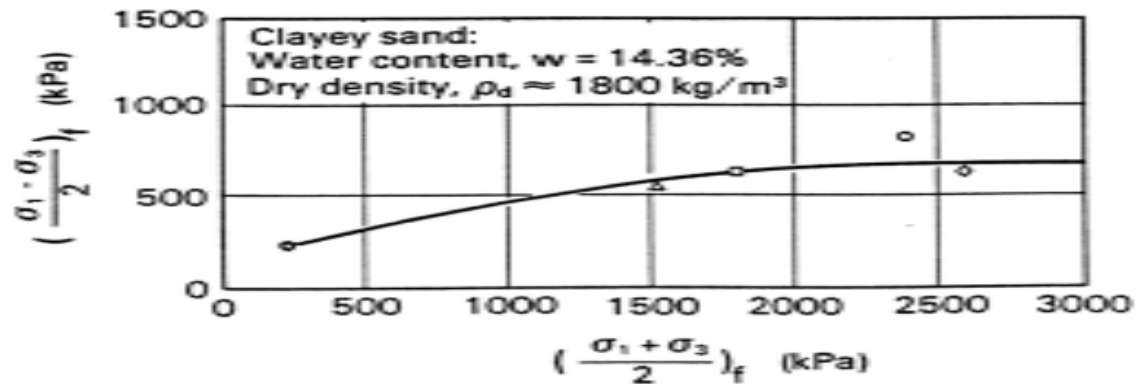




**Results of a series of confined and unconfined compression tests on a compacted clayey sand compacted to a *high* density**



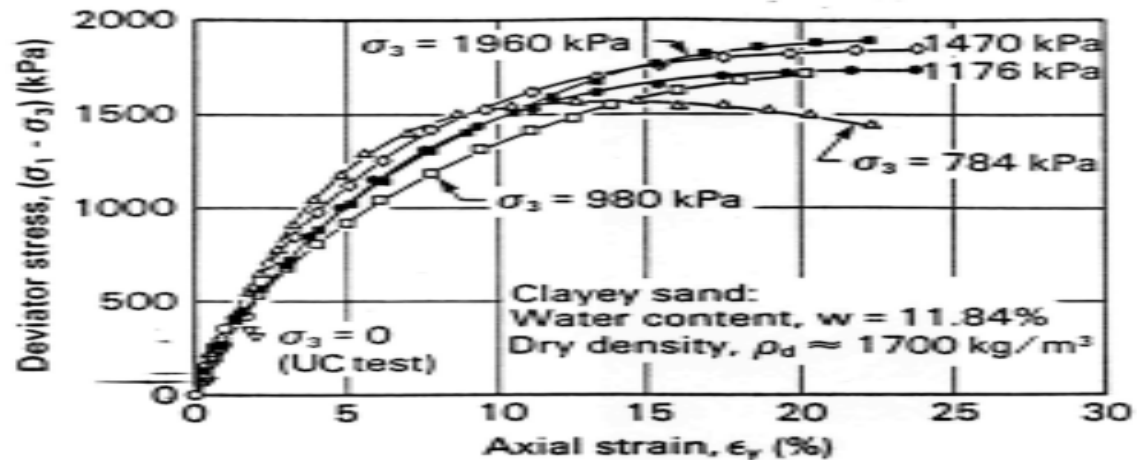
(a) Deviator stress versus strain for various confining pressures



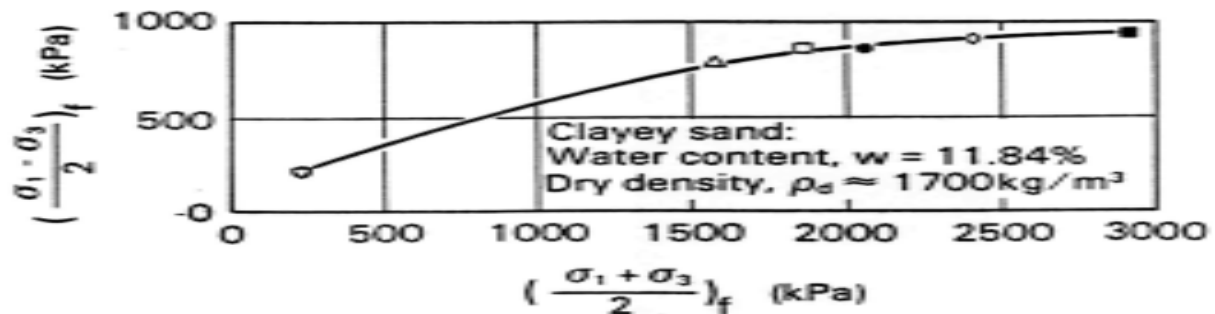
(b) Total stress point envelope

Undrained triaxial and unconfined compression tests on a clayey sand compacted to a high density (from Chantawarangul, 1983)

**Results of a series of confined and unconfined compression tests on a compacted clayey sand compacted to a *low* density**



(a) Deviator stress versus strain for various confining pressures



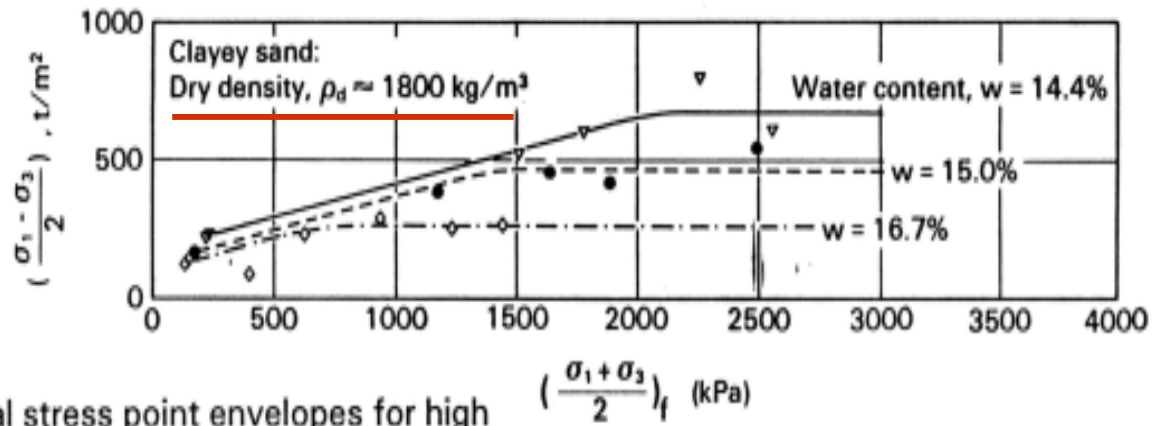
(b) Total stress point envelope

Undrained triaxial and unconfined compression tests on a clayey sand compacted to a low density (from Chantawarangul, 1983)

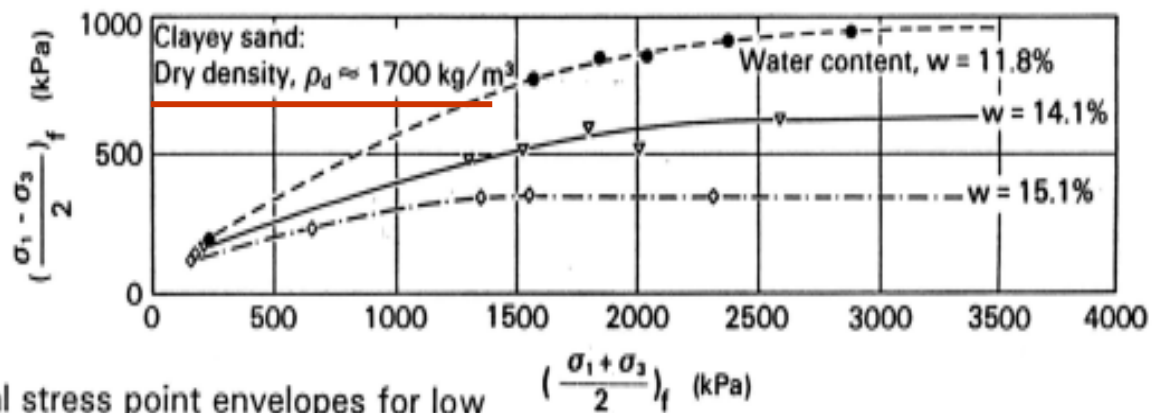


**Effect of  
confinement  
on the  
undrained  
compressive  
strength**

**Undrained and unconfined compression tests**

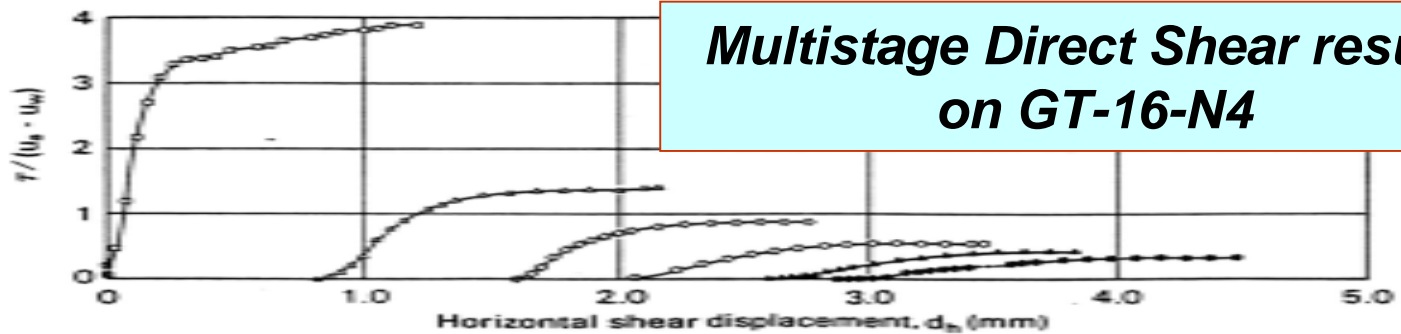
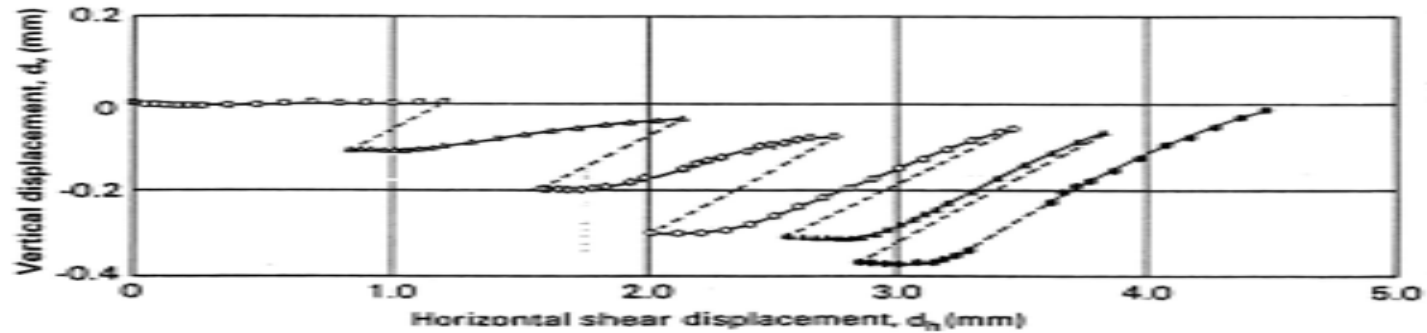
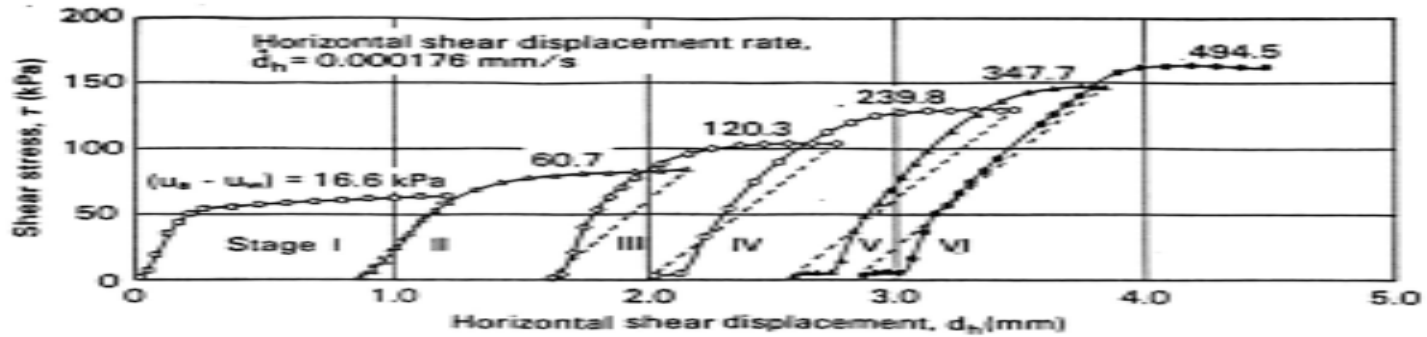


(a) Total stress point envelopes for high density specimens



(b) Total stress point envelopes for low density specimens

Total stress point envelopes obtained from undrained triaxial and unconfined compression tests (from Chantawarangul, 1983)

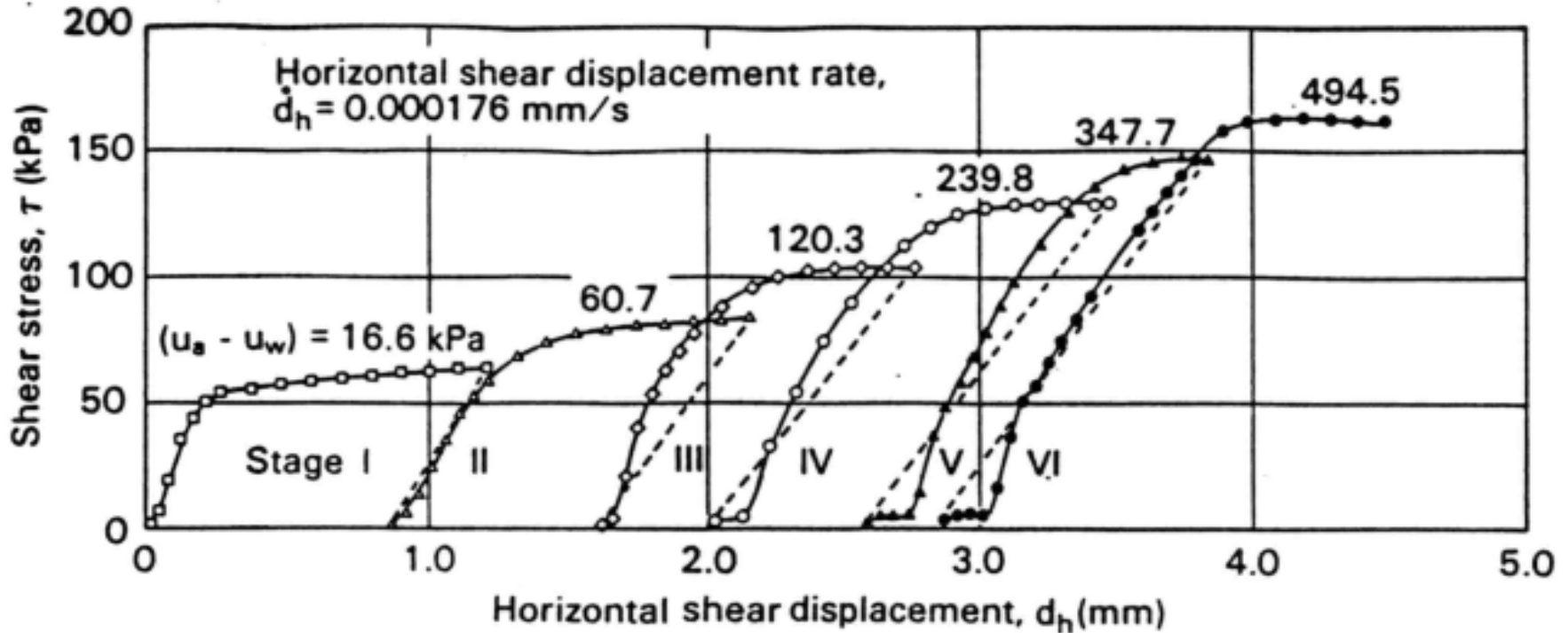


**Multistage Direct Shear results on GT-16-N4**

Multistage direct shear test results on unsaturated glacial till specimen no. GT-16-N4 (from Gan, Fredlund and Rahardjo, 1987)



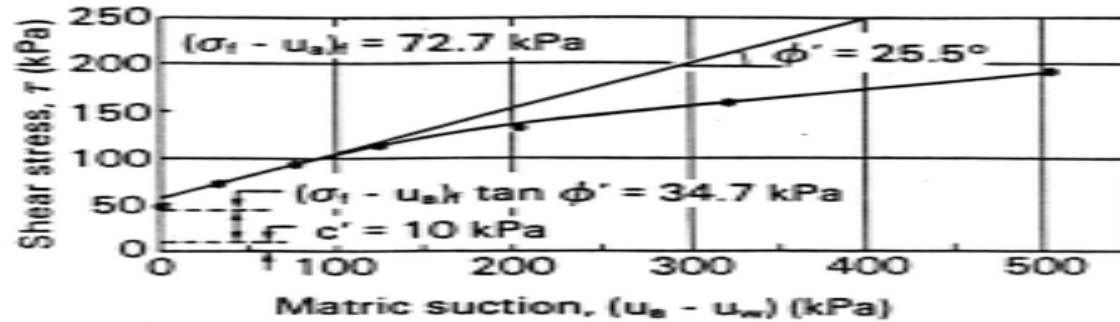
**Multistage Direct Shear results  
on GT-16-N4**



**Note the continual increase in strength with an increase in applied matric suction**

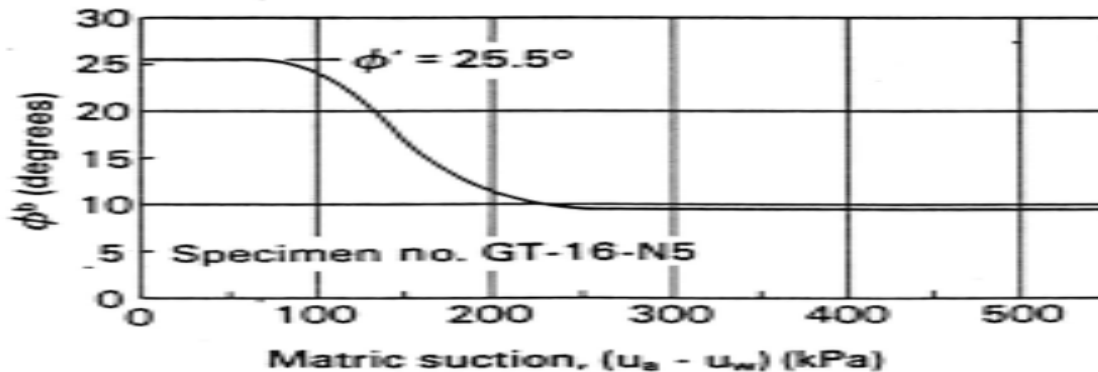


## Nonlinear shear strength versus matric suction



- (a) Failure envelope on the  $\tau$  versus  $(u_a - u_w)$  plane

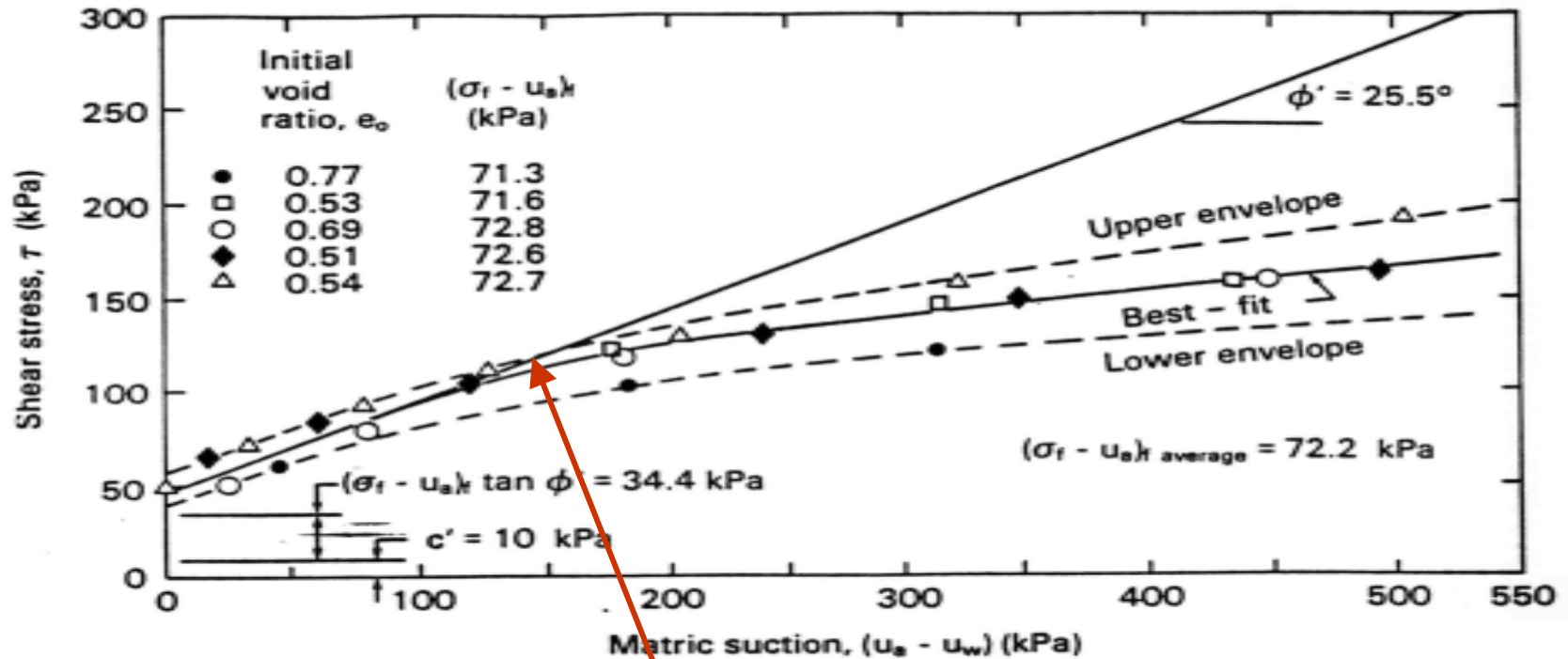
*Interpretation for  $\phi^b$  on GT-16-N5*



- (b) Relationship between the  $\phi^b$  values and matric suction

Failure envelope obtained from unsaturated glacial till specimen no. GT-16-N5 (from Gan and Fredlund, 1988)

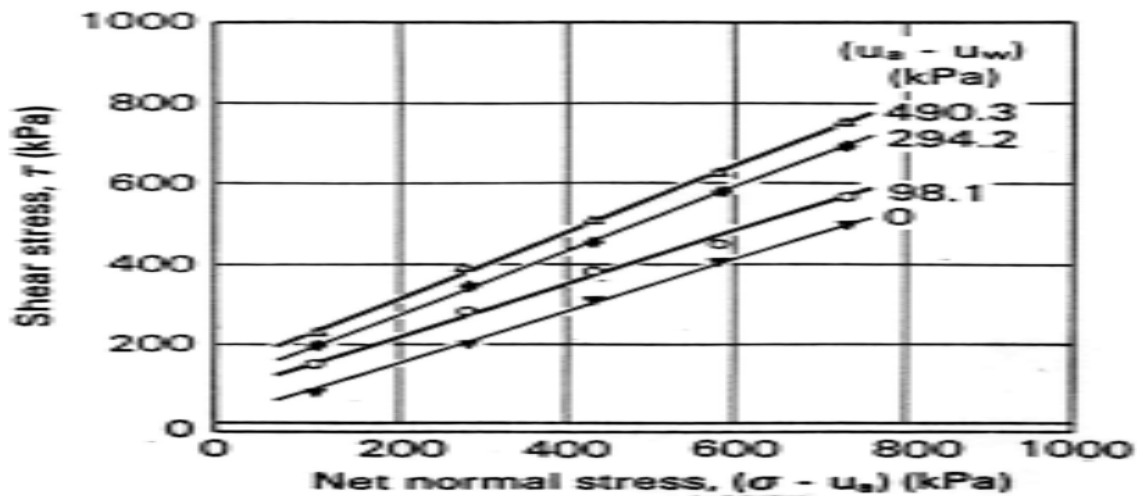
## Results of a series of Direct Shear tests on glacial till



(a) Failure envelopes on the  $\tau$  versus  $(u_a - u_w)$  plane

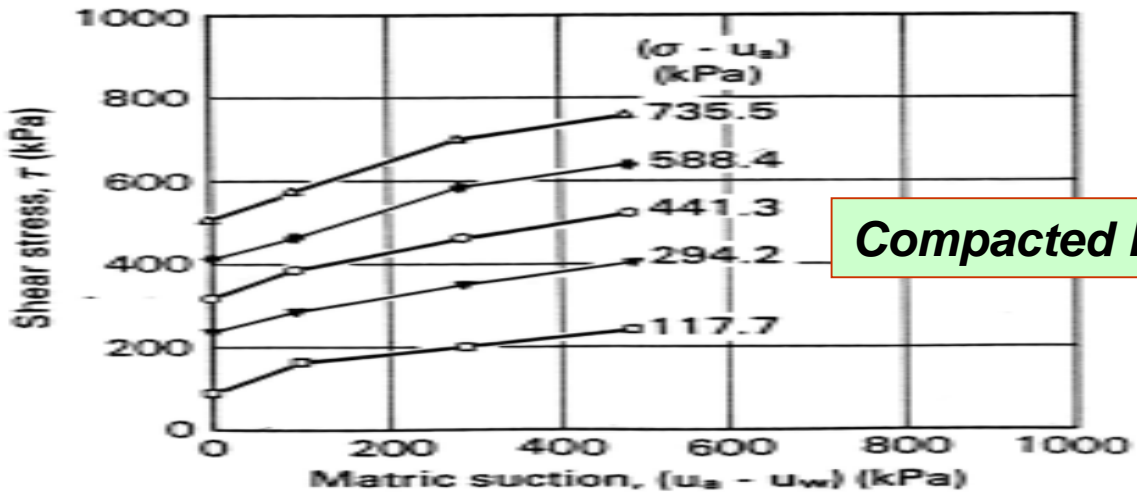
**Commencement of desaturation of the soil**

**Linear on the  
Net Normal  
Stress plane**



(a) Horizontal projections of the failure envelope onto the  $\tau$  versus  $(\sigma - u_a)$  plane

**Curved on the  
matric  
suction plane**



**Compacted Red clay**

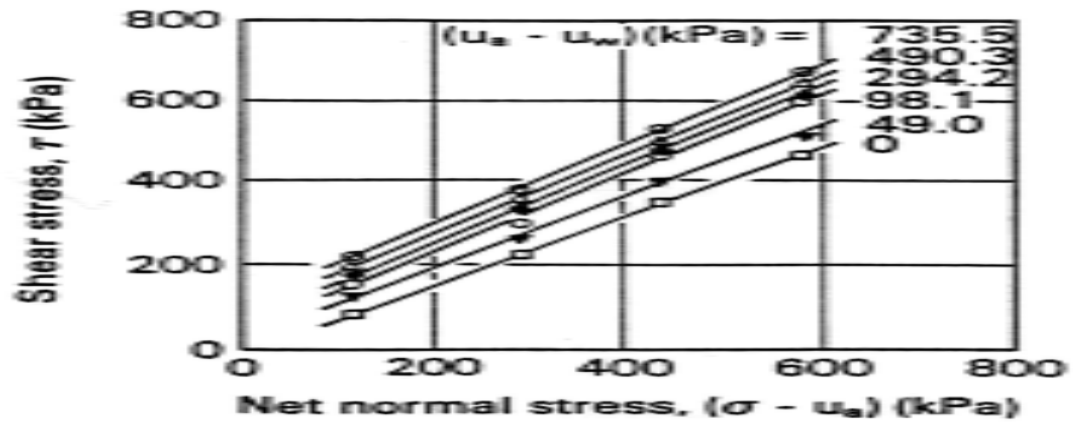
(b) Horizontal projections of the failure envelope onto the  $\tau$  versus  $(u_a - u_w)$  plane

Direct shear tests on compacted red clay of Guadalix de la Sierra (from Escario and Sáez, 1986)





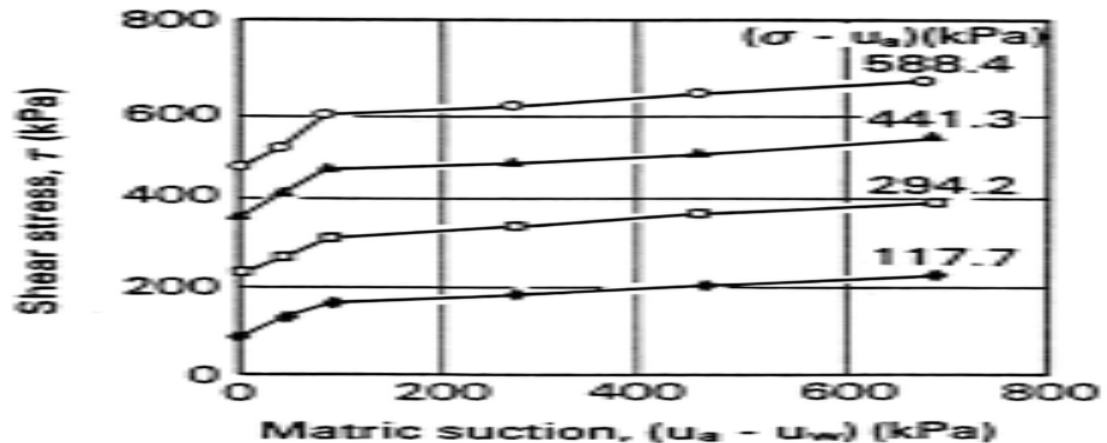
**Linear on the  
Net Normal  
Stress plane**



(a) Horizontal projections of the failure envelope onto the shear stress versus  $(\sigma - u_a)$  plane

**Compacted Madrid  
clayey sand**

**Curved on  
the matric  
suction plane**



(b) Horizontal projections of the failure envelope onto the shear stress versus  $(u_a - u_w)$  plane

Direct shear tests on compacted Madrid clayey sand (from Escario and Sáez, 1986)

