

A photograph of a large-scale construction project at sea. Several tall cranes are visible on the left side, and a long, low structure, possibly a bridge or pier, is under construction in the middle ground. The background shows a hazy coastline with mountains under a blue sky with scattered clouds. The text is overlaid in bright yellow on the upper half of the image.

# **MAINTENANCE PLAN FOR 100 YEARS OF THE SERVICE LIFE ON NEWLY CONSTRUCTED STRUCTURE**

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# INTRODUCTION

- To keep durability of structures during their service life is very important, in the view points not only of keeping their require performance but also of economical and environmental conservation sides, such as reduce of construction wastes.

# INTRODUCTION

## On marine concrete structure

- it is difficult to keep its durability for 100 years or more only by protection ability of concrete itself.
- In some case, use of additional protection system is required in design
- Especially in the important structure. a preventive maintenance may need to be installed during the service life.

# Contents of Presentation

Introduce a structure in which the preventive maintenance concept are installed in its design concept, and actual activity for maintenance will be started from the initial stage of the service, that is the first trial on the real structure in Japan.

- **Outline of Structure**
- **Durability Design**
- **Maintenance Program**
- **Monitoring System for Chloride Attack**
- **Conclusive Remarks**

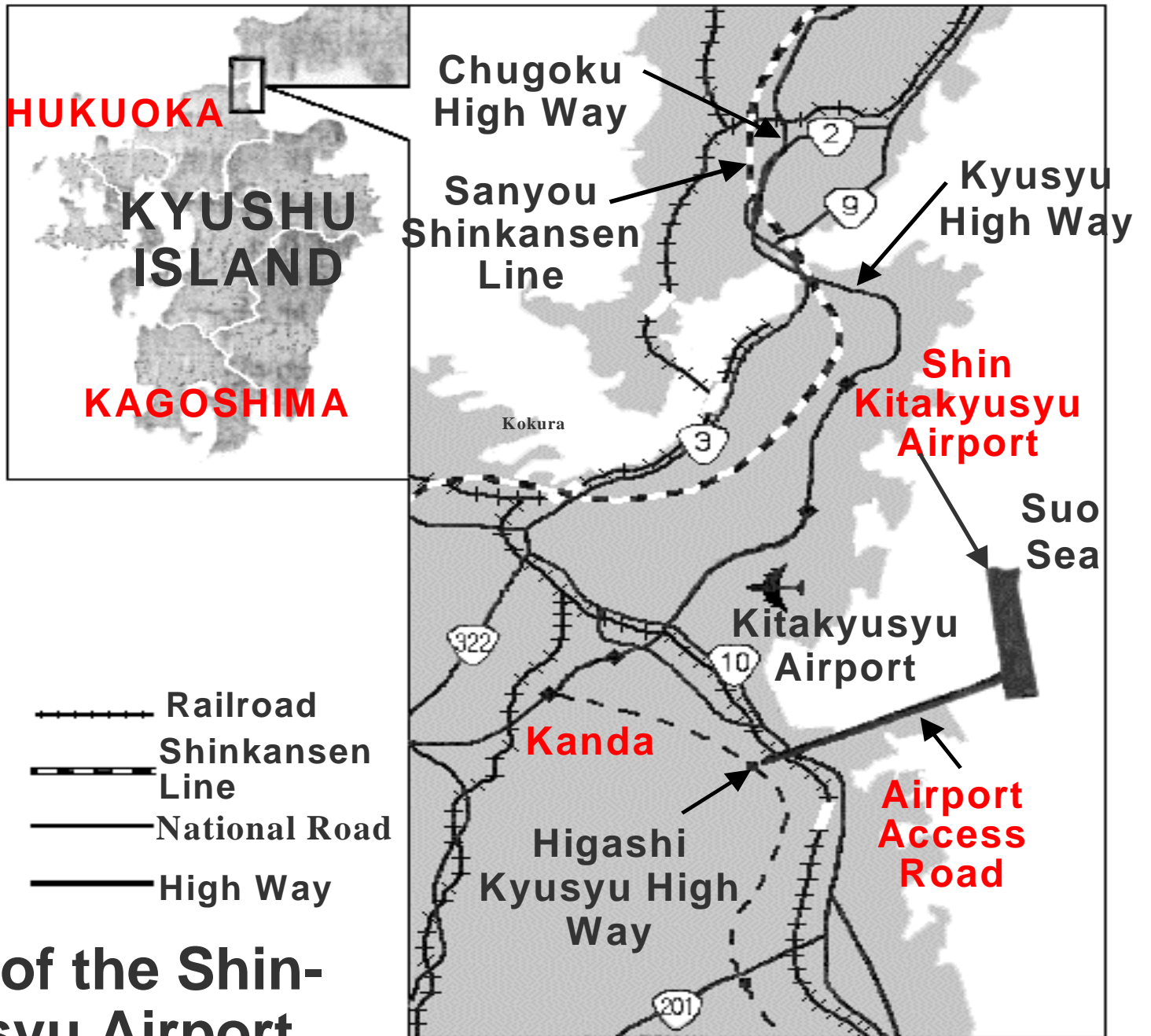
# Out Line of Structure

- **Objective structure:**

Concrete piers in a access bridge of  
Shin-Kitakyusyu airport

# Shin-Kitakyusyu Airport

- Now under construction **as a substitute for Kitakyusyu Airport** which capacity have been overflowed for 2million residents in Kitakyusyu city and the surrounding regions.
- Being scheduled to **open in March 2005**



**Location of the Shin-Kitakyusyu Airport**

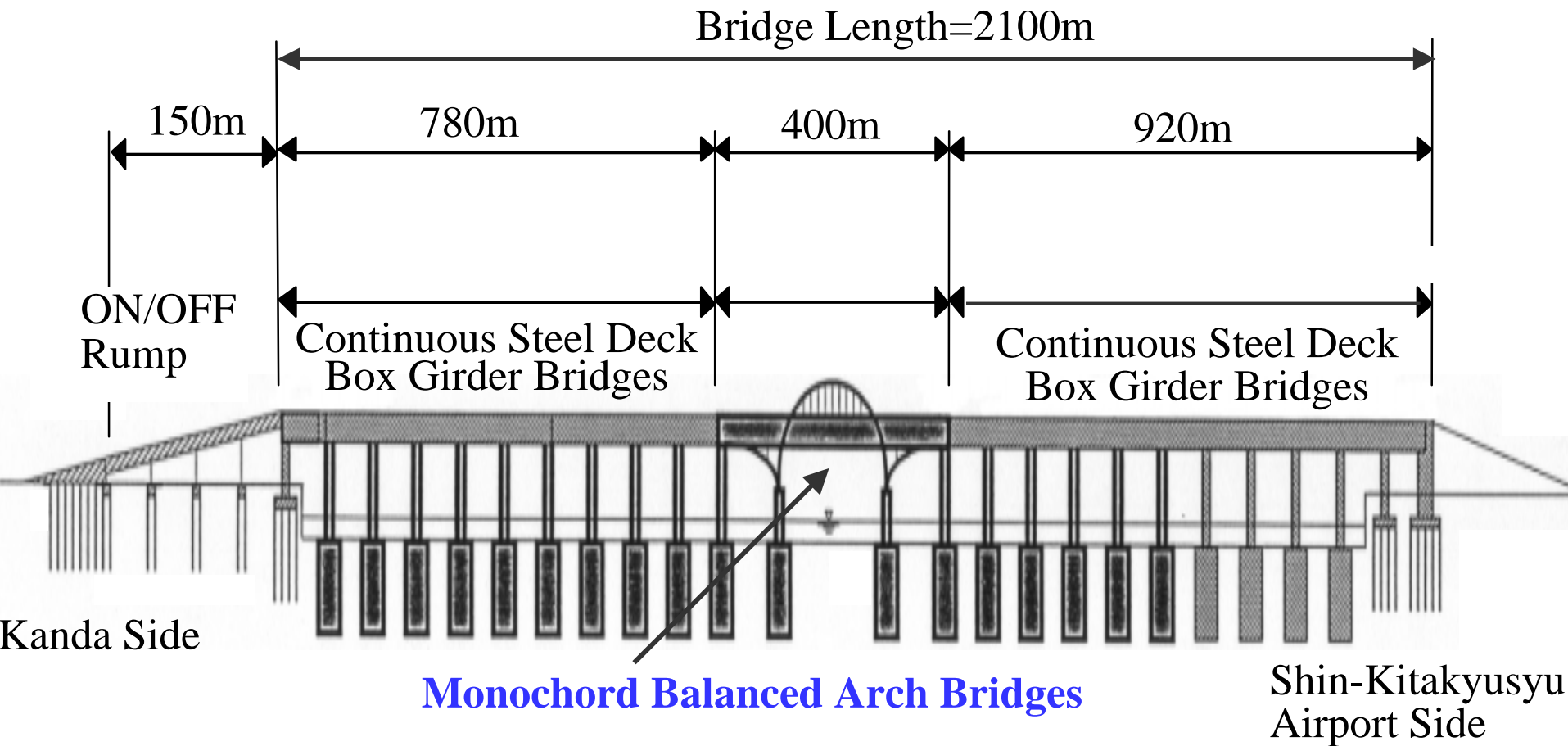
# Shin-Kitakyusyu Airport

- Being constructed **on 370 hectares of a reclaimed island in sea about 2km off the east coast** of Kitakyusyu City
- Being connected with the land (coast) by an access bridge having 2.1 km of a total length



# Shin-Kitakyusyu Airport

- The access bridge that is constructed across the sea is very important structure, because of **the only way to access from the land to the airport**
- **High durability is required for the bridge structure** to keep its required performances and functions during the service life



## Outline of the Access bridge

# Overview of Access Bridge

- **Total length: 2,100m**

- **Superstructure:**

  - Central bridge:**

    - 400-m length of steel monochord balanced arch bridge

  - Approach Bridge(at the land side):**

    - 780-m length of 10-span continuous steel slab box girder bridge

  - Approach Bridge(at airport island side):**

    - 920-m length of 11-span continuous steel slab box girder bridge



**Main Bridge in the Access Bridge**

# Overview of Access Bridge

- **Substructure:**

- Total numbers of 24 of reinforced concrete piers (typical size of cross-section: 6m by 9m)

- / **22 RC piers in offshore**

- / 2 RC piers on the land

- 1 of reinforced concrete abutment

- **Foundations:**

- Open caisson type of foundation using steel pipe sheet pile

- **Design service life : 100years**



**Piers of the Access Bridge**

# Overview of Access Bridge

“Committee on Design and Construction of  
the Access Bridge  
to Shin-Kitakyusyu Airport”

Established by Fukuoka Prefecture Office  
in 1996



“Sub Committee on Concrete Structure”  
(Chairman: Prof. Matsushita, Kyushu Univ.)

- Working from 1996 to 2002 -

To discuss mainly about **the durability of RC  
piers** in the offshore environment

# Durability Design of RC Piers in Access Bridge



# Characteristic of RC Pier

- **Exposed to the severe marine environment**
- **Very important structure**
- **Design service life is 100 years**

# Basic Consideration of Durability Design of RC Piers

- **Piers are constructed in the severe maritime conditions:**
  - ⇒ **Need to ensure the durability against chloride attack**

# Durability Design of RC Piers

-- Examination in view point of durability against chloride attack --

**Basically according with the JSCE Standard Specification (1996 version)**

## ■ Mixture proportion of concrete

$f'_{ck}$ N/mm <sup>2</sup>	Gmax mm	slump cm	Air %	W/C %	s/a %	Unit weight (kg/m <sup>3</sup> )				
						W	C	S	G	AE
27	20	8 ± 1.5	4.5 ± 1.5	45	40	149	330	717	1165	1.17

## ■ Execution-related measures

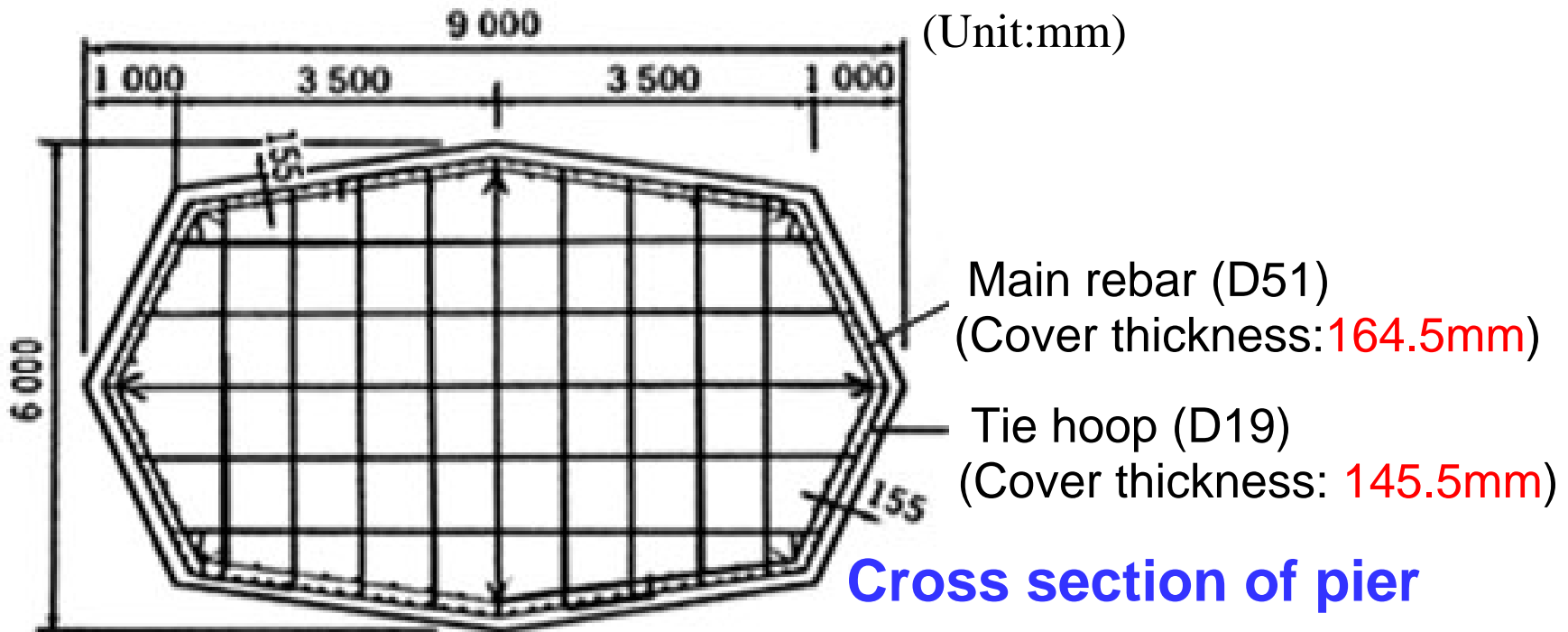
/ Use of permeable formworks

# Durability Design of RC Piers

-- Examination in view point of durability against chloride attack --

**Basically according with the JSCE Standard Specification (1996 version)**

- **Concrete cover thickness : More than 10.5cm**



# Durability Design of RC Piers

**JSCE Standard Specification (1996 version)**  
was not based on the performance-based  
design concept but on prescription-based  
design concept



Durability of structure is decided only by  
structural detail and material performance  
prescribed in the specification

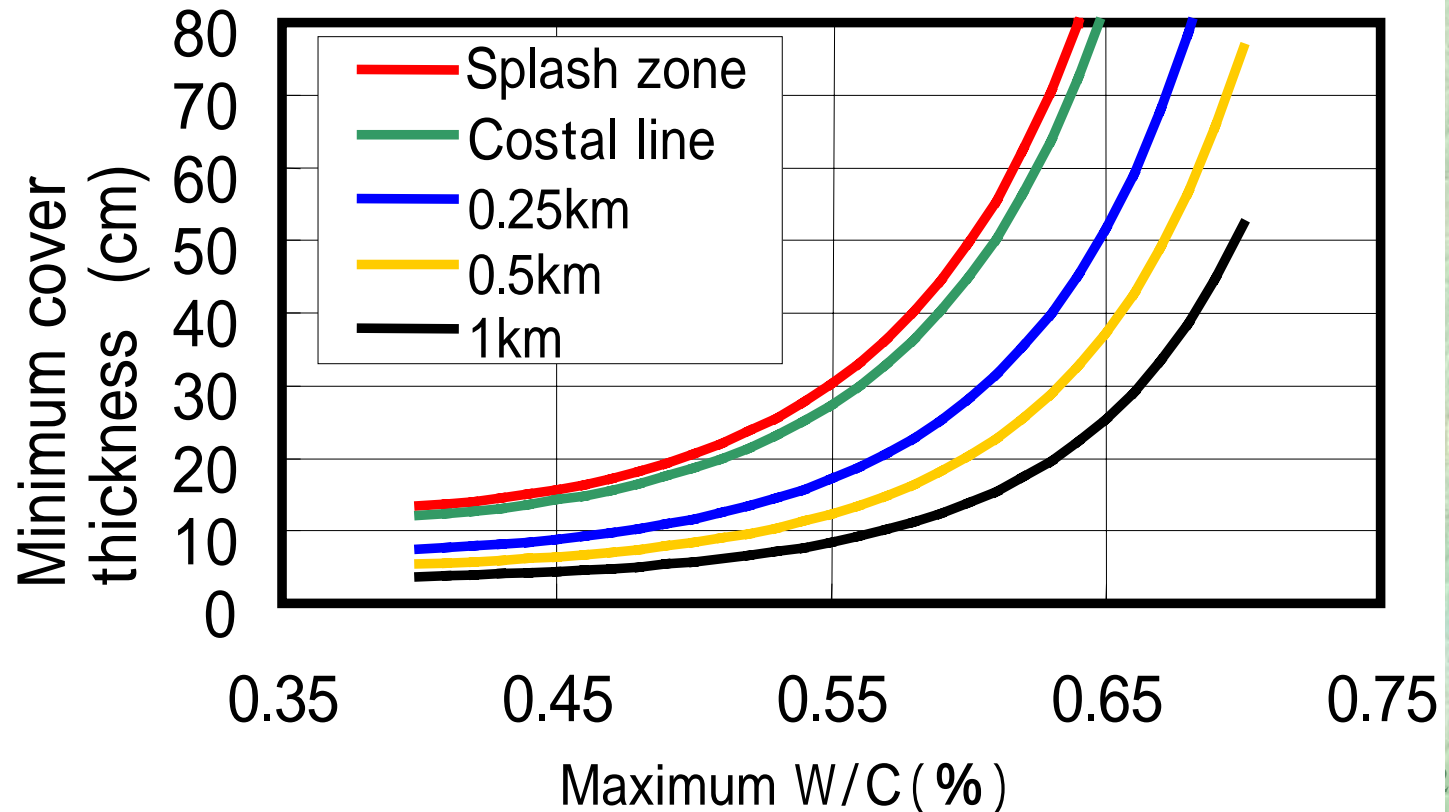


**Design conditions were confirmed to satisfy  
JSCE Standard Specification (1999 version)**  
which is based on the performance-based  
design concept

# Relationship between minimum cover thickness and maximum W/C required for durability satisfying JSCE Specification(1999)

## Design condition

**Blast furnace slag: 50%, Design service life: 100 years**





**Arrangement Condition of Reinforcing Steel**

# Durability Design of RC Piers

-- Examination in view point of AAR --

## Preliminary survey

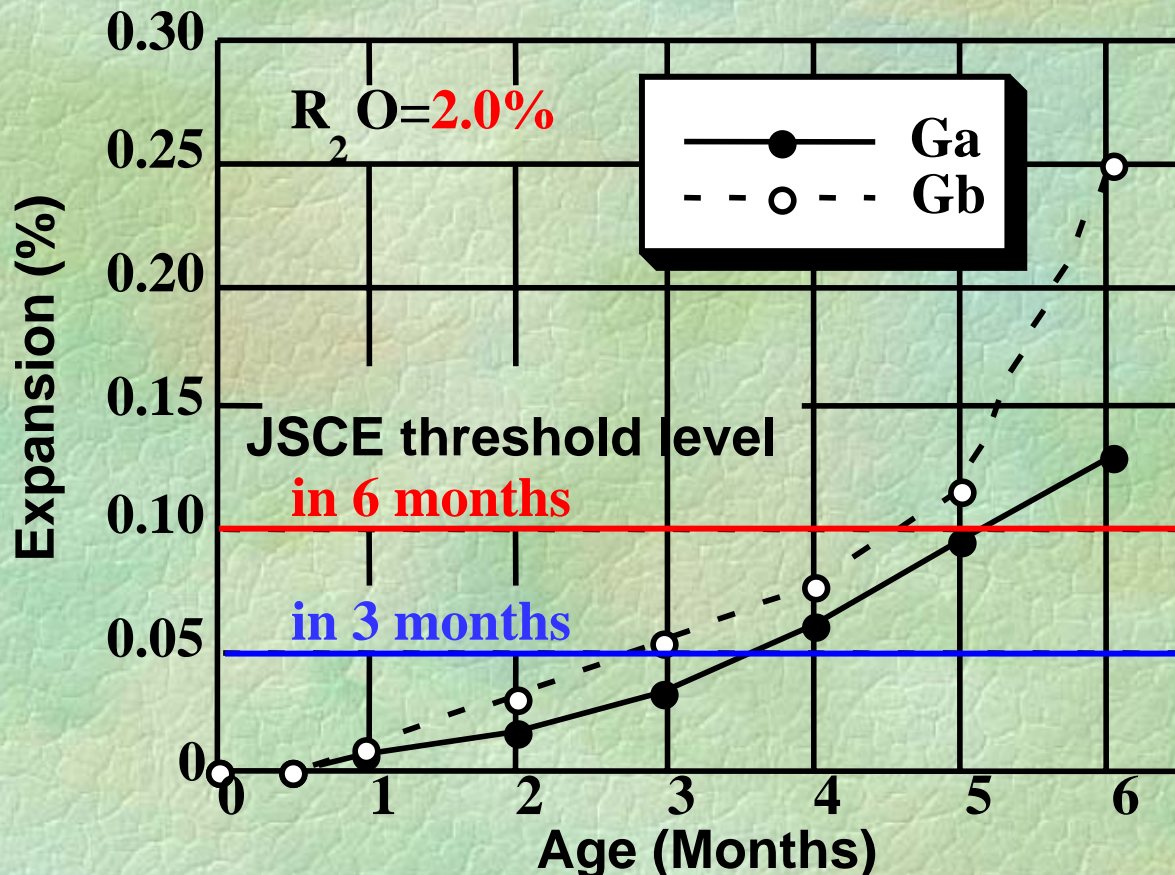
- 15 types of aggregate were sampled from concrete mixing plants near the construction site.
- All of aggregate was judged as innocuous under test condition of the chemical method or the mortar bar method specified by JIS
- Some of aggregate showed abnormal expansion under higher alkali concentration than the critical value in the JIS test method



# Durability Design of RC Piers

-- Examination in view point of AAR --

## Preliminary survey



Test result of mortar-bar method under higher alkali concentration

# Durability Design of RC Piers

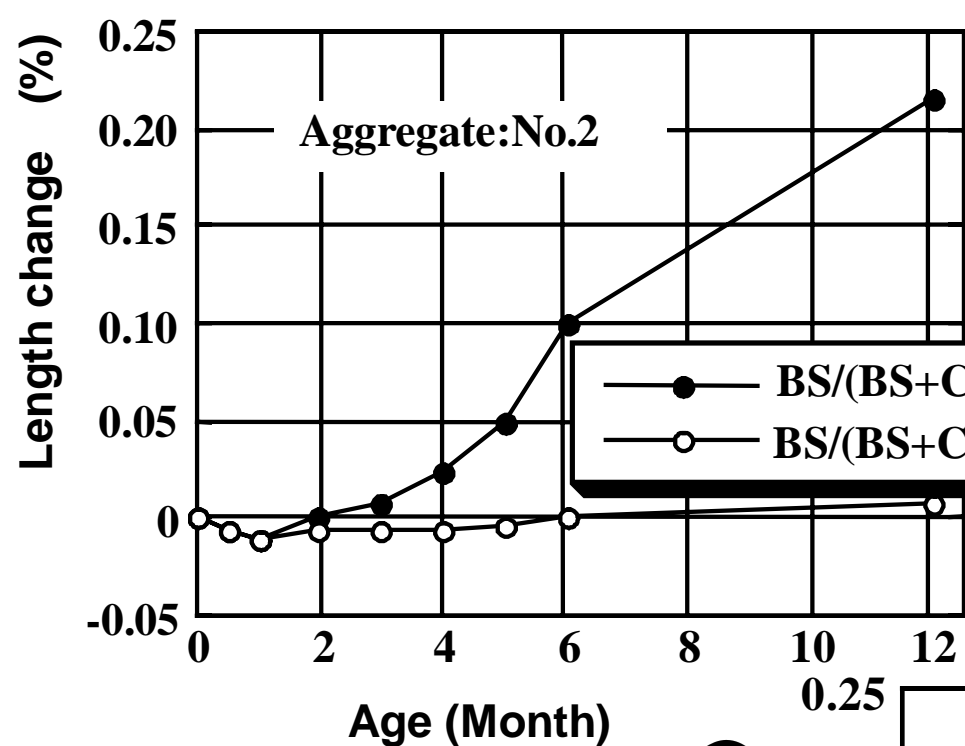
## -- Examination in view point of AAR --

- In marine environment, a lot of alkali ions such as  $\text{Na}^+$  and  $\text{K}^+$  penetrates and concentrates in concrete
- It is difficult to select only the innocuous aggregate using for the pier, because the marketing system of aggregate is very complex.

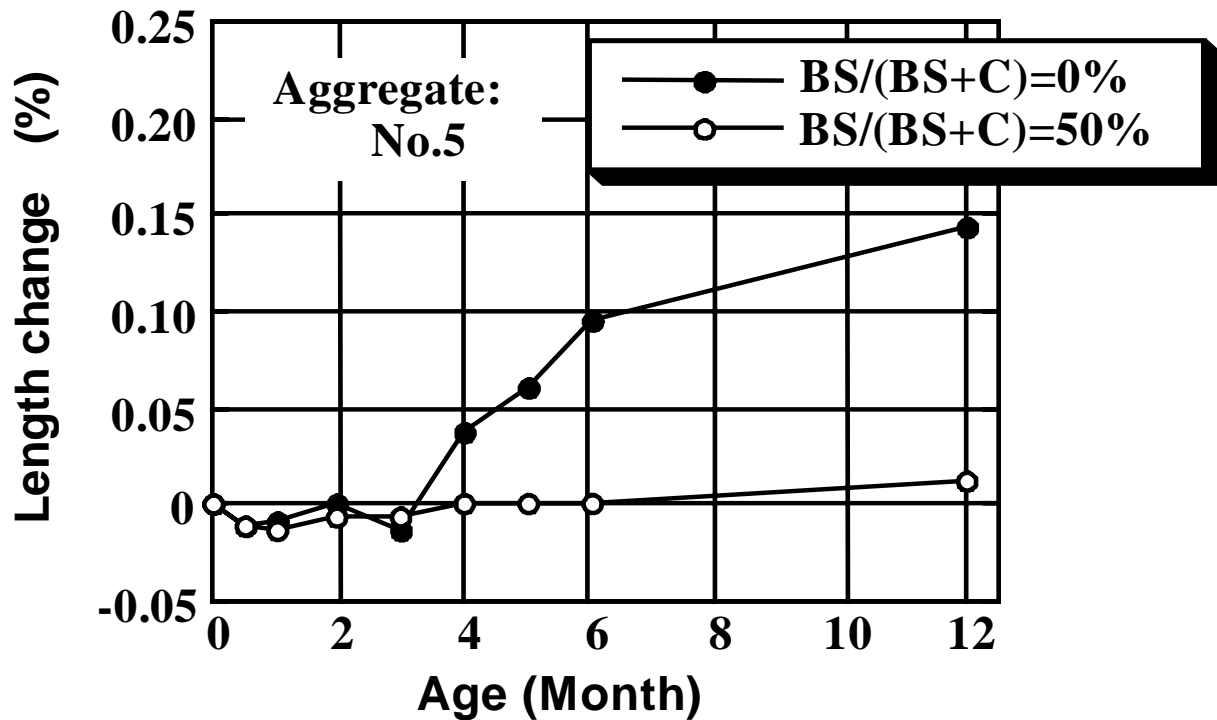


### Countermeasure

**Use of Ground granulated blast-furnace slag  
in replace of 50% or more of cement**



**Inhibiting effect of  
GGBF slag on  
AAR**



# Durability Design of RC Piers

-- Examination in view point of thermal crack --

## Result of thermal stress analysis

- The size of the pier makes it difficult to eliminate thermal cracking completely



## Realistic countermeasure

- To control the crack width within its threshold value in consideration of the durability of pier.

# Durability Design of RC Piers

Examination in view point of thermal crack --

## Countermeasure for the thermal crack (1)

Use of low-heat Portland cement with GGBF slag at replacement ratio of 60 %

## Additional Measure for thermal crack

Use of heat insulation formwork made of 5mm thickness of foamed polystyrene



## Heat Insulation Formwork



**Appearance of Thermal Crack on the Pier**

# Durability Design of RC Piers

-- Examination in view point of thermal crack --

## Countermeasure for the thermal crack

If thermal cracks occur during the execution, they should be repaired in accordance with the following manners;

### In the case of 0.2mm or more of crack width:

➡ Repair by both **epoxy resin injection** in the crack and **epoxy coating** on the concrete surface

### In the case of less than 0.2mm of crack width:

➡ No repair



# Maintenance Program for RC Piers in Access Bridge

**-- Preventive Maintenance Concept --**

# Maintenance Program for the Pier

## **Risk of deterioration in the durability design**

- It is difficult to control generation of the AAR crack and thermal crack perfectly
- Difficulty of repair after deterioration occurs during service



**“Preventive Maintenance Concept”  
is Required**



## **Characteristic of the pier**

- Very important structure
- Design service life is 100 years

# Maintenance Program for the Pier

## -- Sub-Committee's Conclusion --

A scheme of durability design of RC piers should be established **in consideration of preventive maintenance activity** performed continuously during its design service life



Such intentional maintenance of newly constructed RC structures is the first case in Japan

# Maintenance Program for the Pier

## -- Preventive Maintenance Concept --

### **Preventive Maintenance Concept**

To keep required performances of the pier during its service life without any loss due to deterioration



- **Maintenance activity should be started from the initial stage of the service**
- **Monitoring system should be introduced to detect the sign of deterioration**

# Maintenance Program for the Pier

-- Preventive Maintenance Concept --

In order to carry out preventive maintenance systematically on the pier during its service life



**“Maintenance Guideline for Reinforced Concrete Piers of Access Bridge to Shin-Kitakyusyu Airport”**

was published by the Sub-Committee  
in 2002



**“Maintenance Guideline for Reinforced Concrete Piers of Access Bridge to Shin-Kitakyusyu Airport”**

# **Maintenance Guideline for Reinforced Concrete Piers of Access Bridge to Shin-Kitakyusyu Airport --Contents--**

## **Part 1: Maintenance Fundamental**

- 1.General
- 2.Required Performance of Pier and Specific Performance Controlled in Maintenance Activity
- 3.Inspection
4. Verification of Performance, Assessment and Judgment
- 5.Remedial Measures
- 6.Records

## **Part 2: Manual for Inspection and Verification**

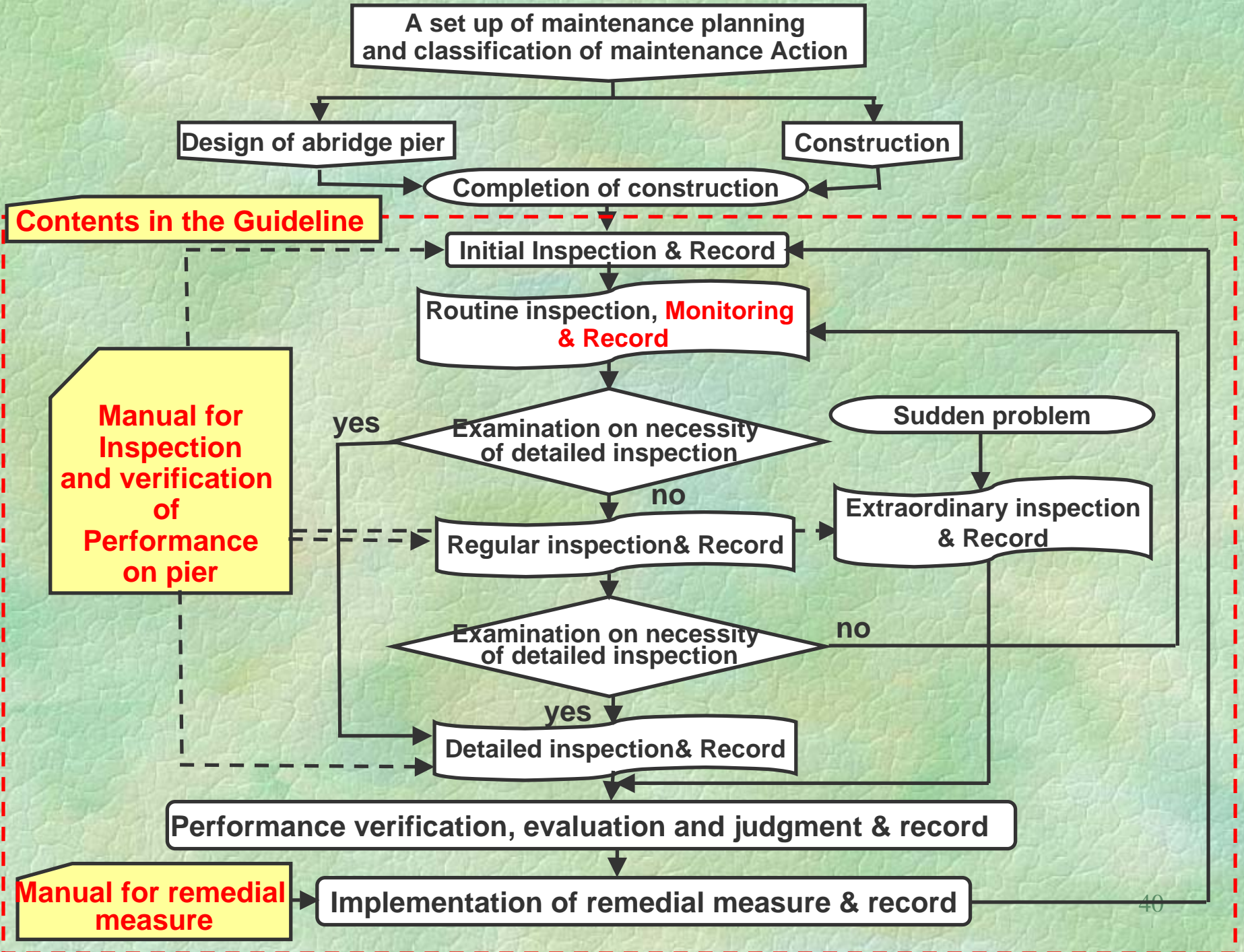
## **Part 3: Manual for Remedial Measures**

Sub-Part 1: Measure for Deterioration Due to Chloride Attack

Sub-Part 2: Measure for Deterioration Due to Alkali-Aggregate Reaction

## **Appendices:**

1. Example for Basic Flow in Maintenance Action for Expected Deterioration Condition on the Pier
2. Grading of Deterioration in Visual Inspection
3. Outline of Monitoring System and Measurement Method





# Maintenance Program for the Pier

## -- Monitoring System --

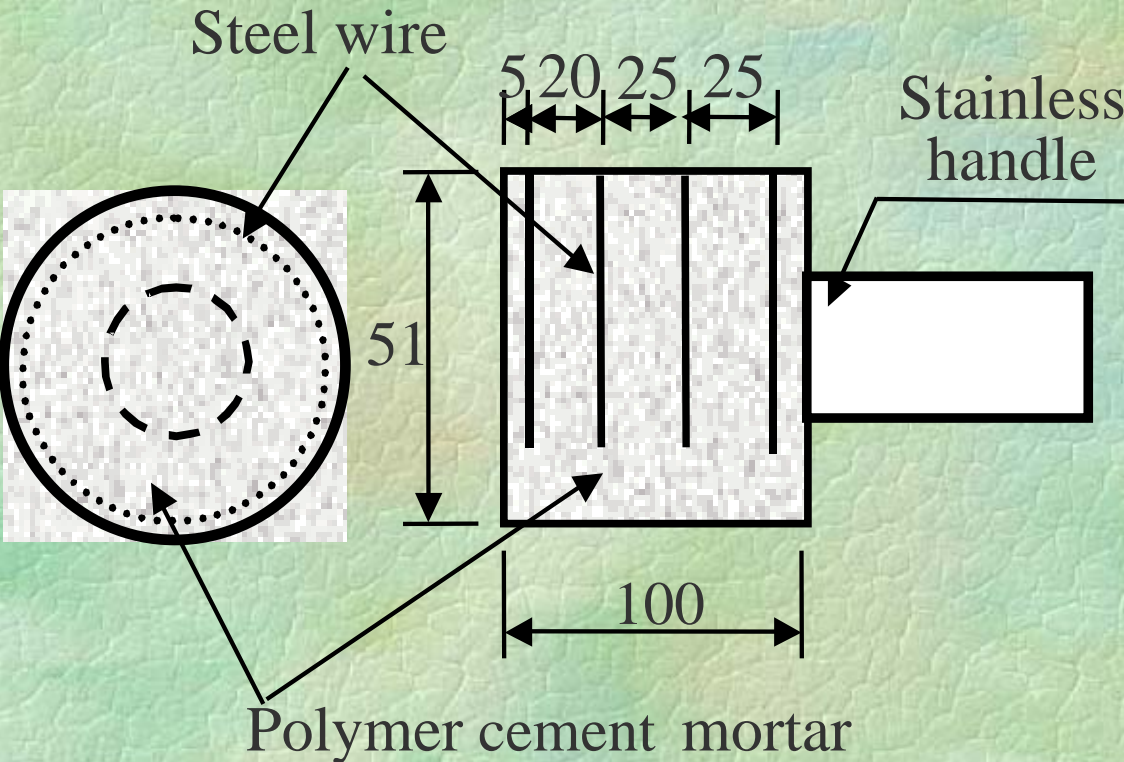
Two types of monitoring system have been installed in the piers constructed in the offshore environment



- Newly developed monitoring sensor to **detect chloride penetration depth into concrete** non-destructively
- Embedded type reference electrode for monitoring natural potential of rebar in concrete **to detect the initiation of reinforcement corrosion**

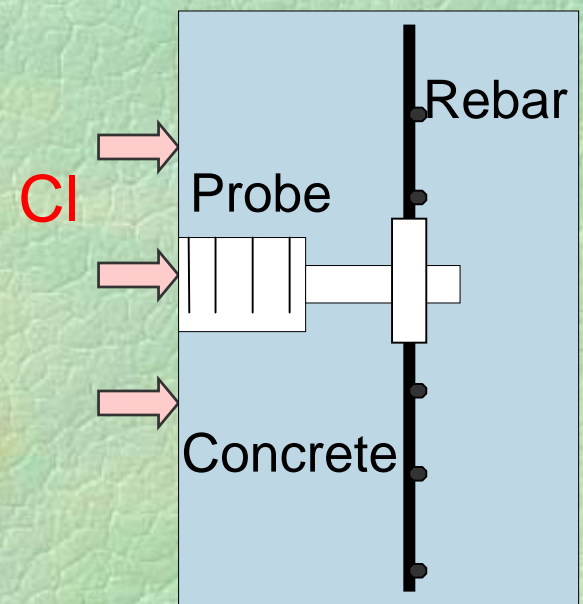
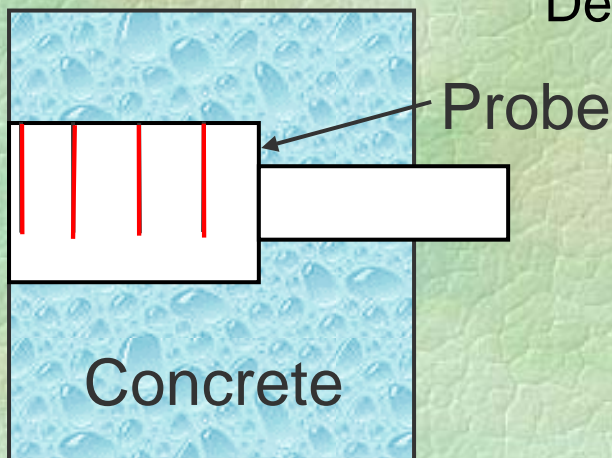
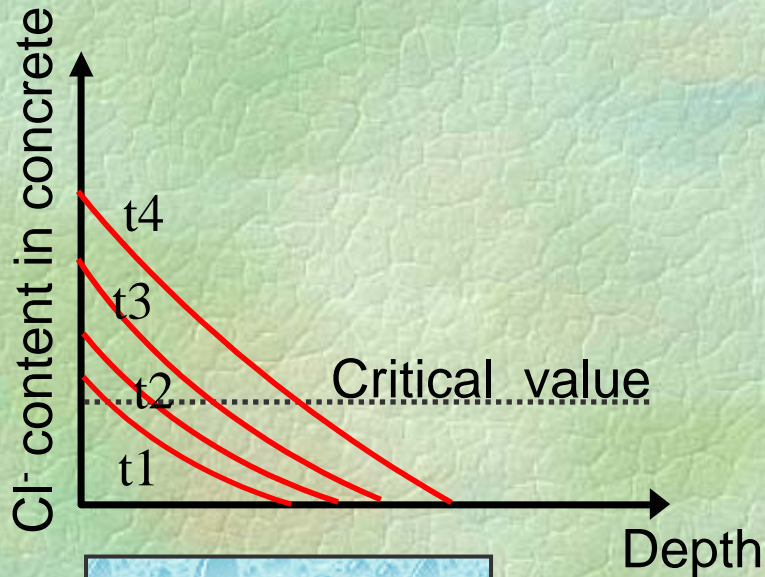
# Chloride Penetration Monitoring Sensor

New sensor system detecting the depth where chloride content is critical level for initiating rebar corrosion



- Four steel wires having 0.1 mm of diameter are set in ditches along the circumference, at distances 5, 25, 50 and 75 mm from the top surface.

# Chloride Penetration Monitoring Sensor



Critical chloride content

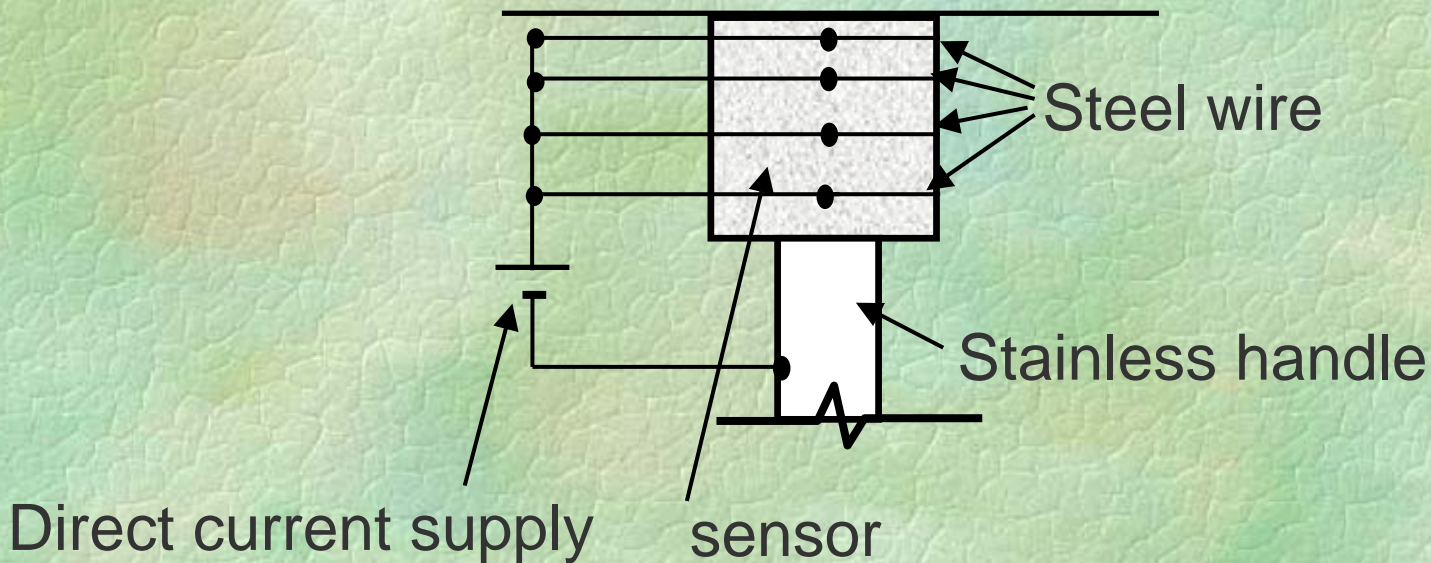
Corrosion of steel wire

The wire breaks

Sharp increment of electric resistance of wire

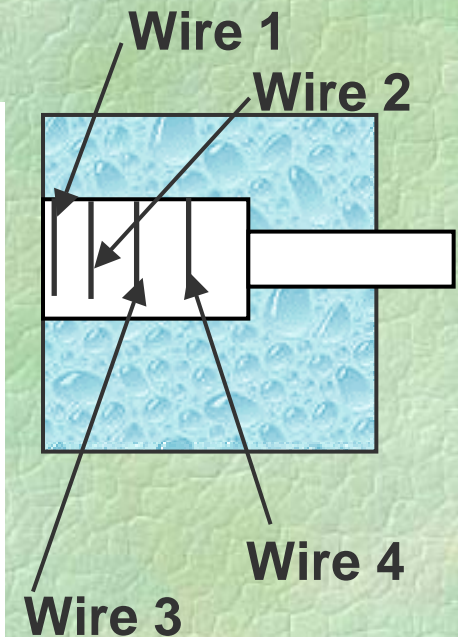
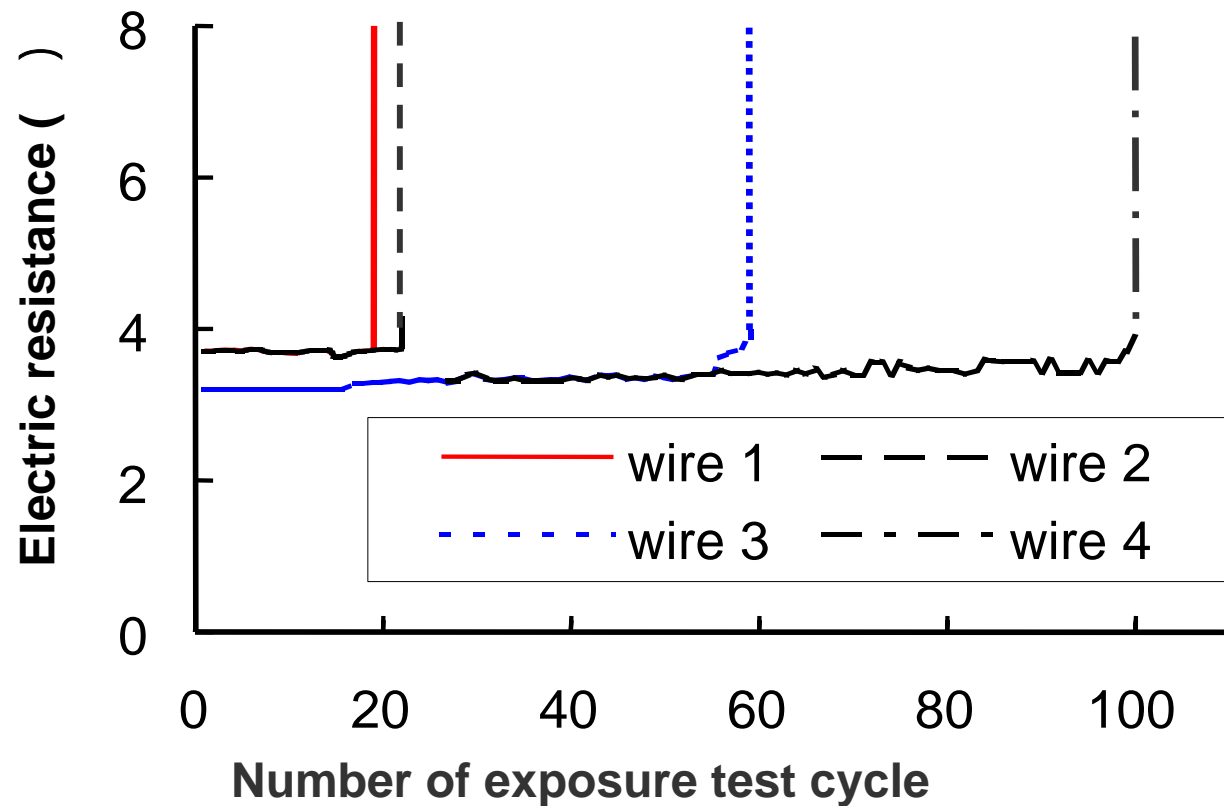
# Chloride Penetration Monitoring Sensor

**For improvement on sensitivity of sensor, small current supply to wires in sensor and potentials of wires are polarized 400 - 450 mV in positive direction**



# Chloride Penetration Monitoring Sensor

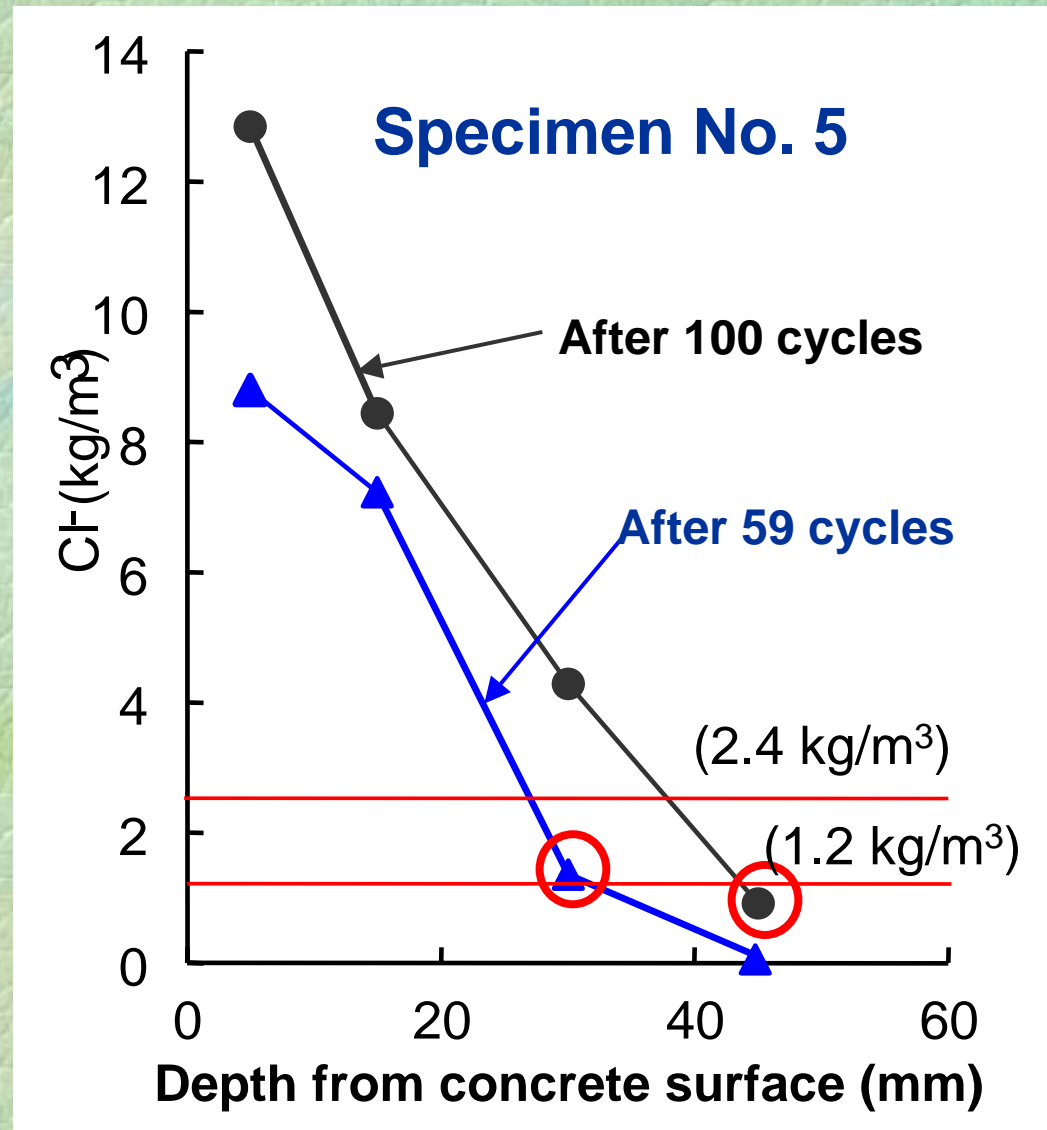
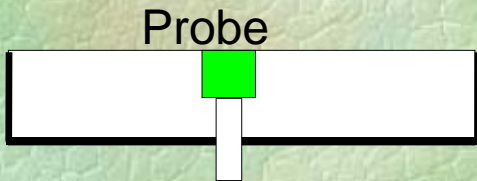
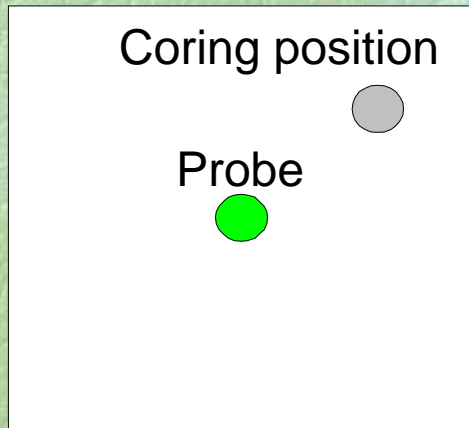
An Example of indoor experiment for examining sensitivity of the sensor



It can be confirmed that electric resistance of the wire set in sensor shows sudden increment at the certain time in order of the depth from the surface.

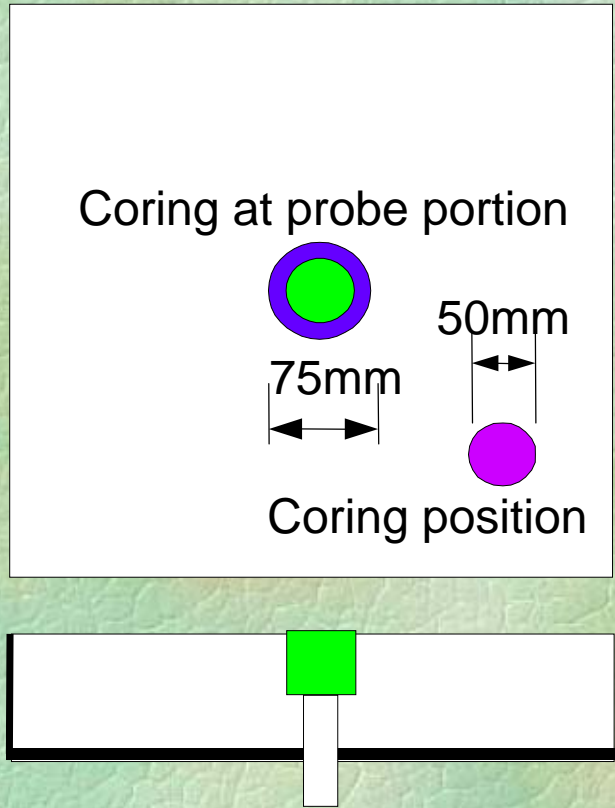
# Improvement on Sensitivity of Probe

## Chloride content in Concrete

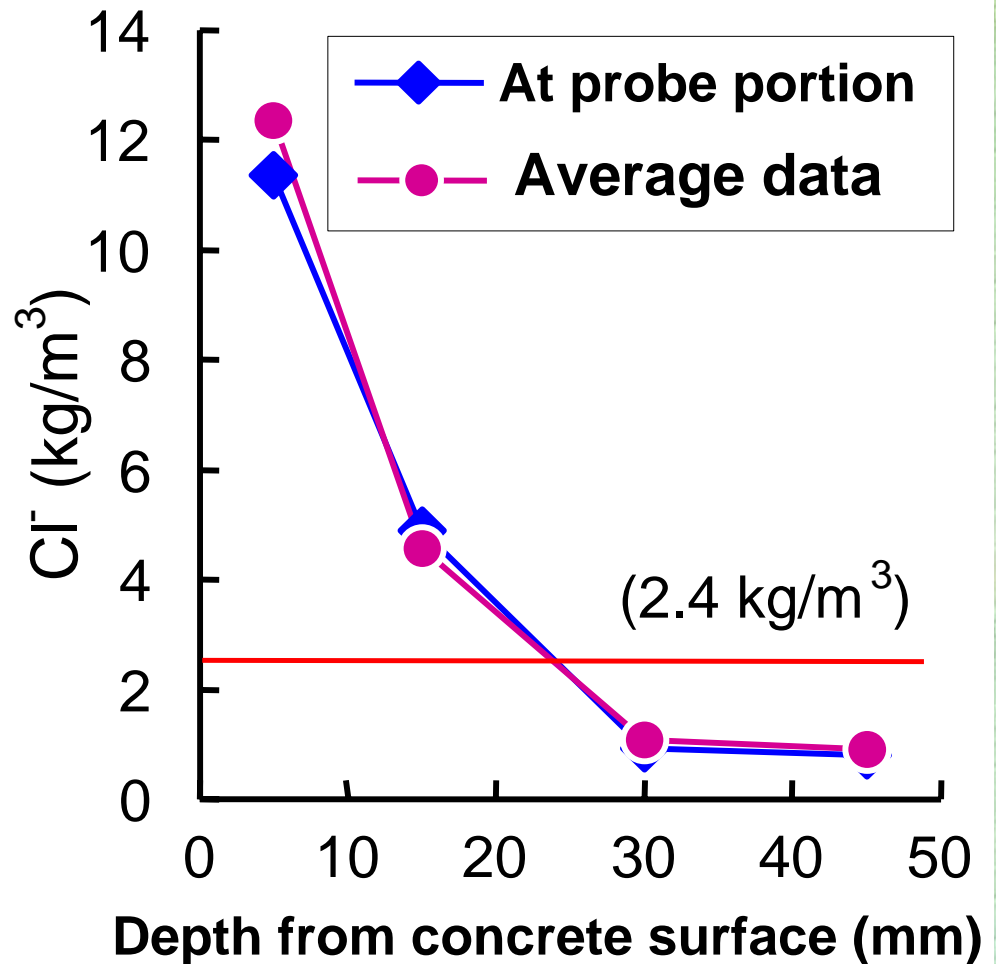


/ The wires broke whenever the chloride content reached about **1.0 kg/m<sup>3</sup>** in surrounding concrete.

# Improvement on Sensitivity of Probe Chloride content in Concrete



Specimen No. 2 ( after 35 cycles)



/ Presence of the probe did not disturb chloride penetration process at concrete near to probe in the specimen No.2, to which any current was not supplied.

# ESTIMATION OF CORROSION START TIME ON REBAR BY USING MONITORING DATA

- Chloride diffusion into concrete can be analyzed macroscopically in accordance with the Fick's second law

$$\frac{\partial C(x,t)}{\partial t} = D \frac{\partial^2 C(x,t)}{\partial x^2}$$

**(Assumption)**

**Diffusion coefficient (D) and chloride concentration at concrete surface (C<sub>0</sub>) are constant respectively.**



$$C_{(c,t)} = C_0 \left[ 1 - \operatorname{erf} \left( \frac{x}{2\sqrt{D \cdot t}} \right) \right]$$



When assuming that critical chloride content for initiating steel corrosion is constant,

$$C_0 \left[ 1 - \operatorname{erf} \left( \frac{x_i}{2\sqrt{D \cdot t_i}} \right) \right] = C_0 \left[ 1 - \operatorname{erf} \left( \frac{x_R}{2\sqrt{D \cdot t_R}} \right) \right] = C_c$$

Corrosion start time ( $t_R$ ) on rebar having  $x_R$  of concrete cover thickness can be estimated by a following equation using the monitoring data of corrosion initiation time ( $t_i$ ) and depth ( $x_i$ ) of wire ( $i$ ).

$$t_R = \left( \frac{x_R}{x_i} \right)^2 t_i$$

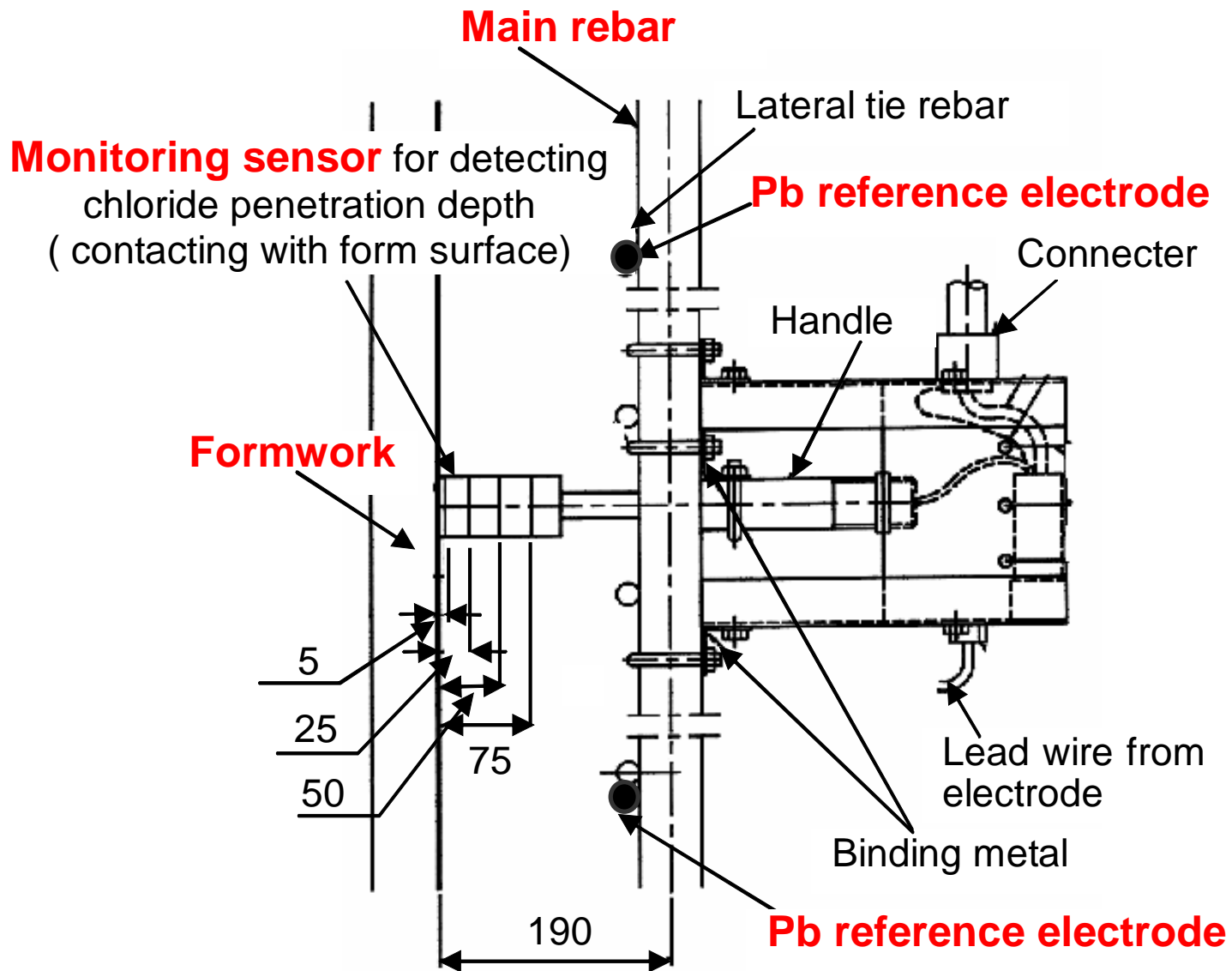
$x_R$  : Cover thickness of rebar  
 $x_i$  : Depth of wire ( $i$ ) corroded  
 $t_i$  : Corrosion start time of wire ( $i$ )

# ESTIMATION OF CORROSION START TIME ON REBAR BY USING MONITORING DATA

- Comparison between actual breaking time of wire(i+1) and its estimation result by using the breaking time data of wire (i)

Cover thickness	Corrosion start time of wire (day)	Corrosion start time estimated by using the result at shallower portion (day)	Ratio between estimation and actual result
15mm	65	-----	---
30mm	177	260	1.47
45mm	366	398	1.09

Relatively accurate corrosion time can be estimated



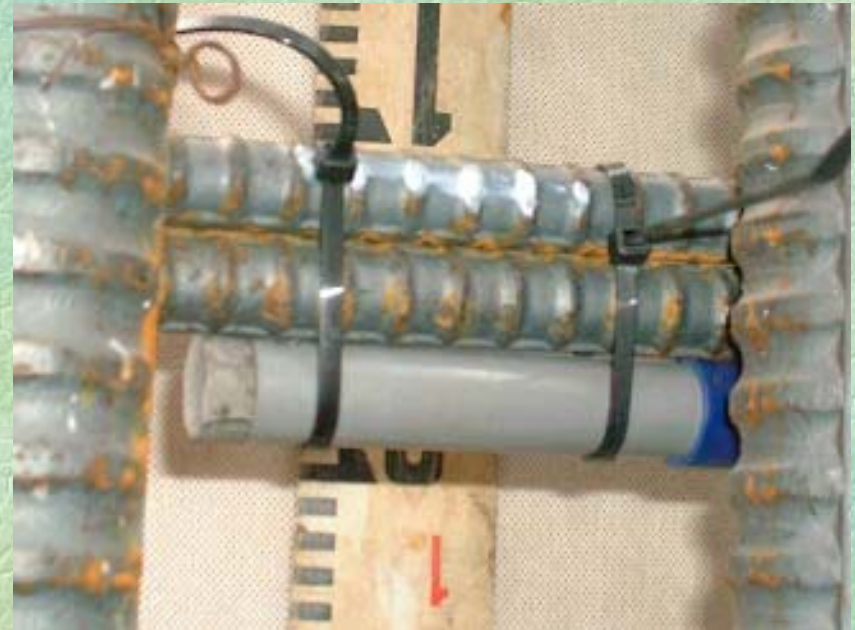
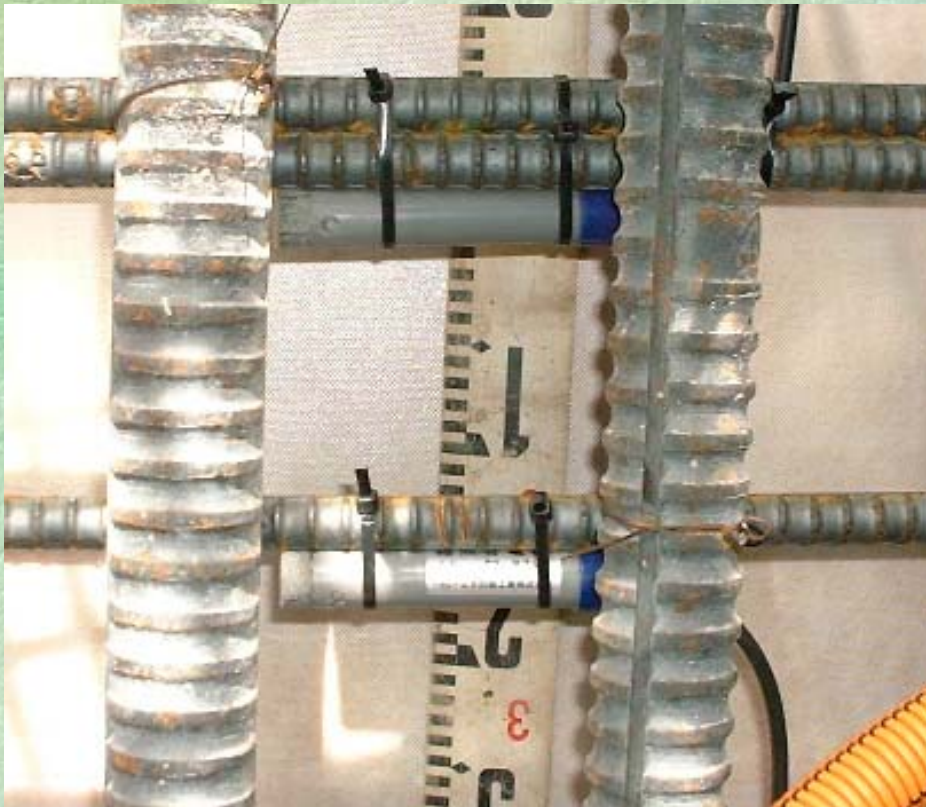
**Outline of Installed Condition of Monitoring Equipment in the Pier**

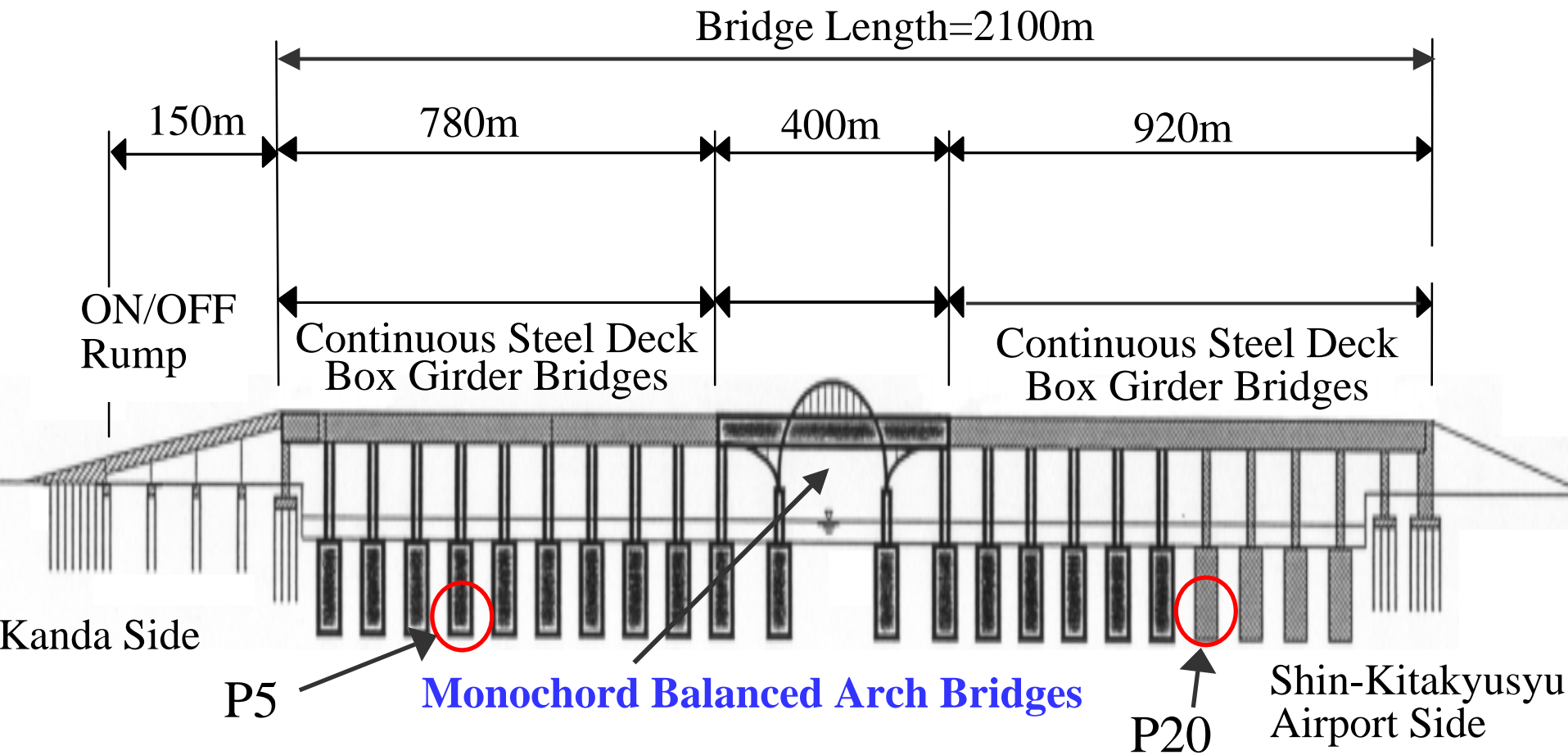


# Monitoring Natural Potential of Rebar by Embedded Type Reference electrode

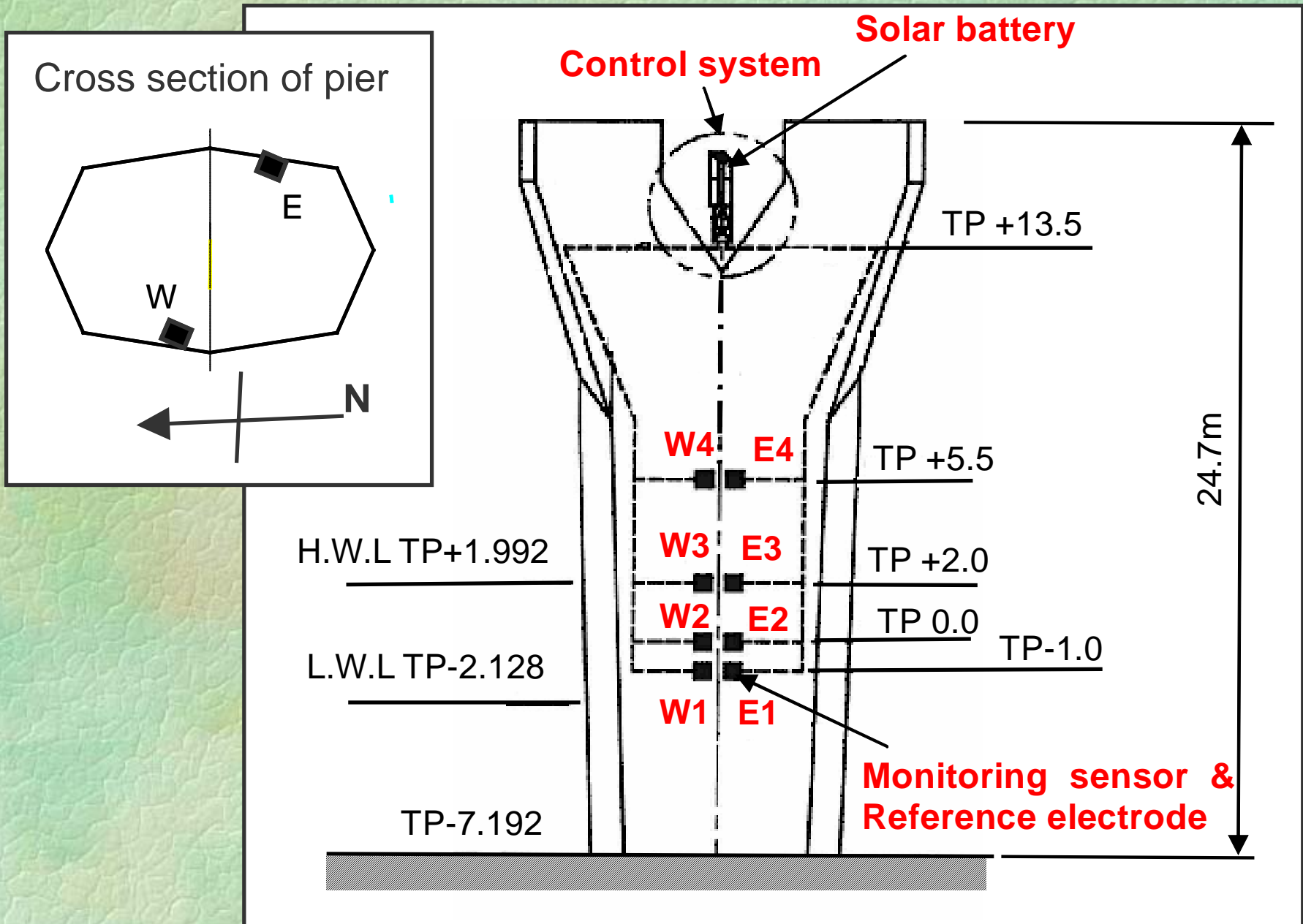
Some number of reference electrodes have arranged in concrete;

- To predict **the initiation in the rebar corrosion**
- To estimate **the macro-cell corrosion** formation

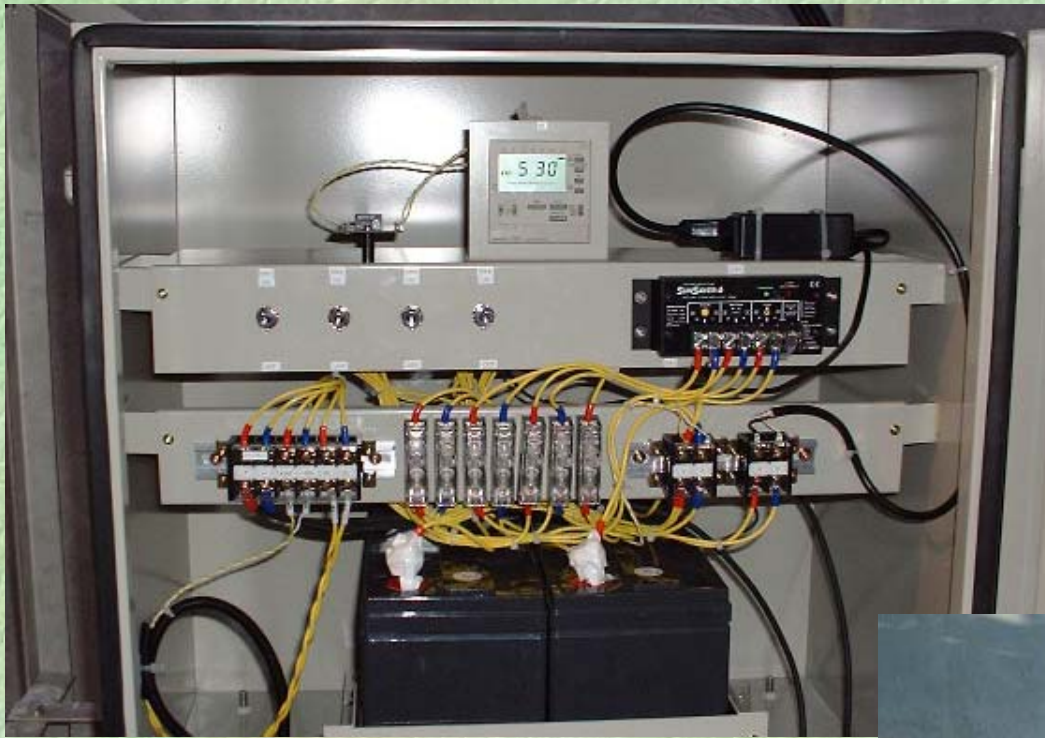




**Location of piers having monitoring systems**



**Positions of the Monitoring System Arranged in Pier**



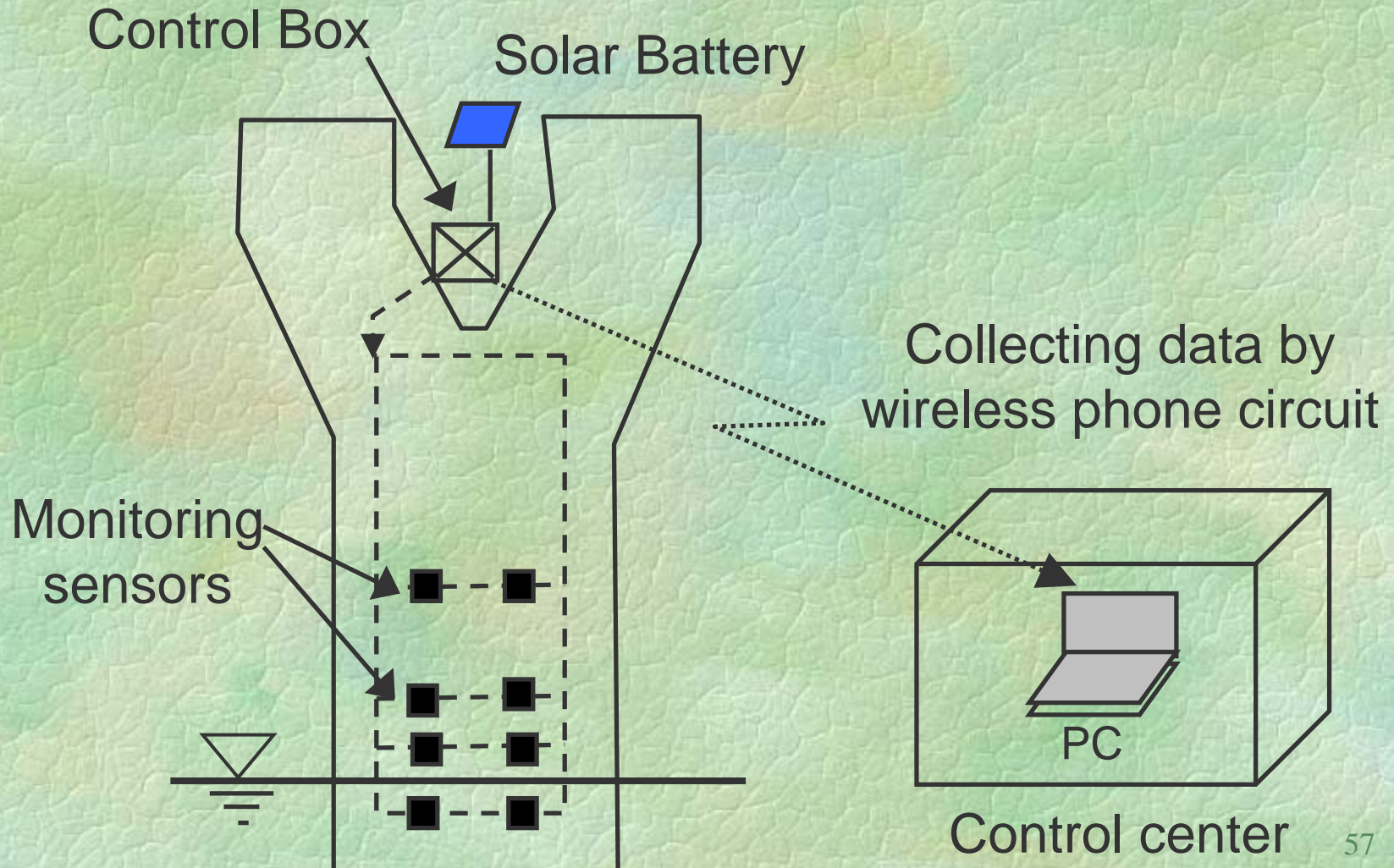
**Control Box**

**Solar panel**





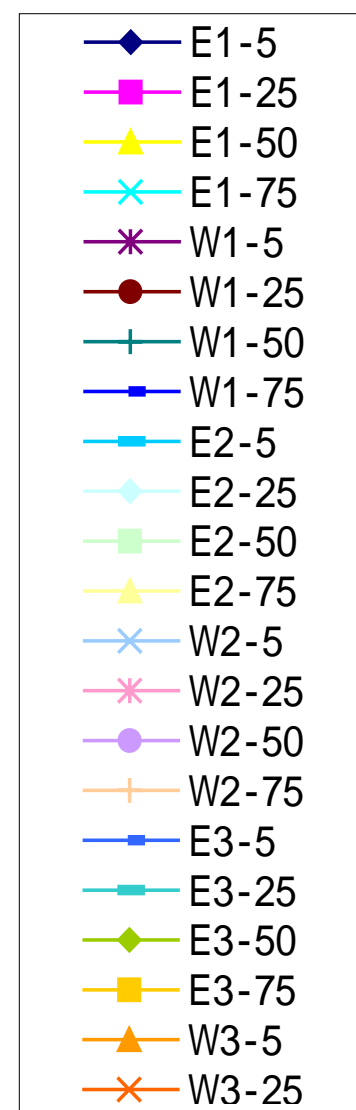
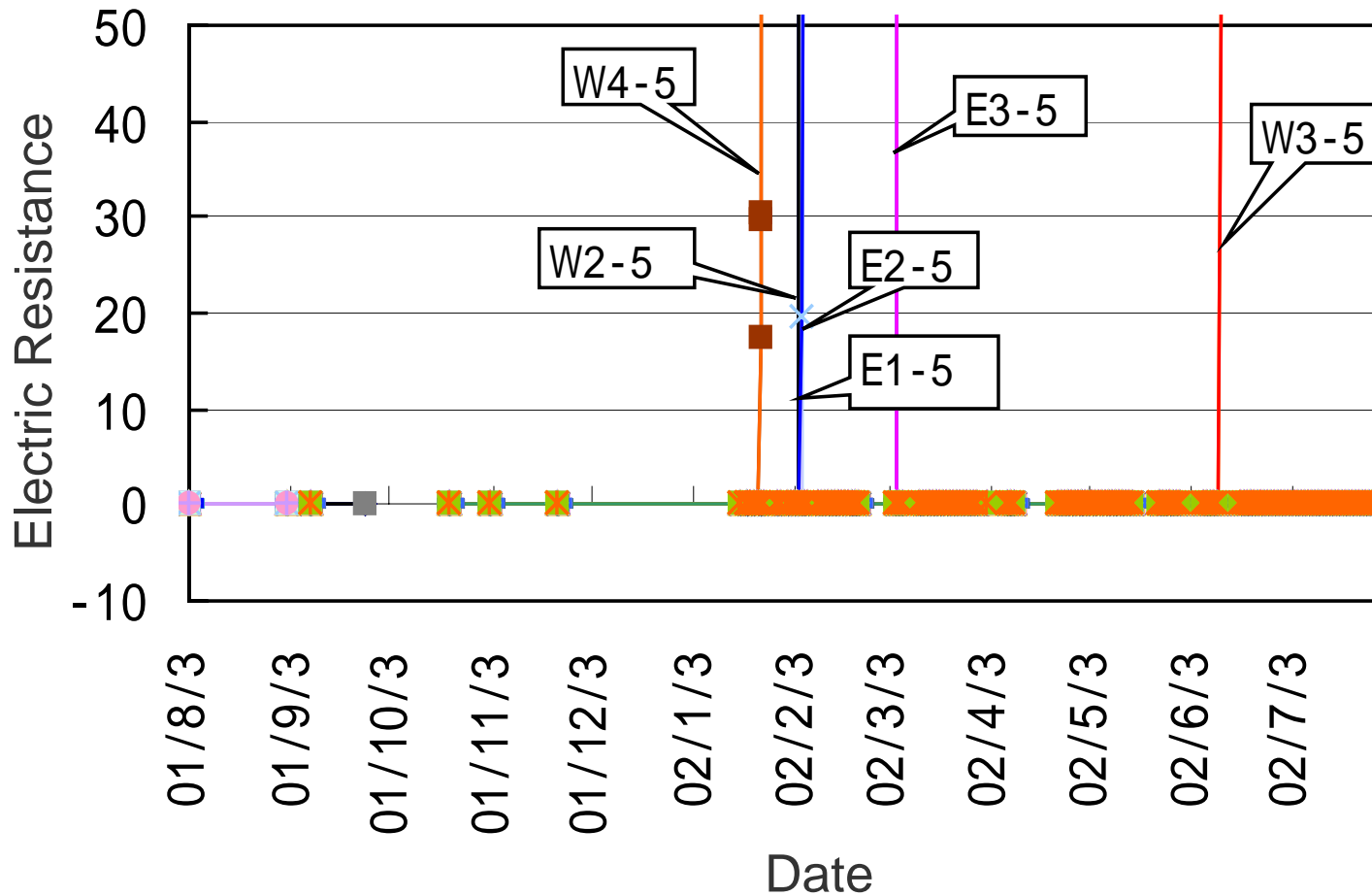
# All of monitoring data can be sent to PC in control center by using wireless phone circuit



# Indication data form chloride penetration monitoring sensors

全体	腐食センサ E1	腐食センサ W1	腐食センサ E2	腐食センサ W2	腐食センサ E3	腐食センサ W3	腐食センサ E4	腐食センサ W4	センサ電位 電源電圧	初期画面	
腐食センサE1		腐食センサW1		腐食センサE2		腐食センサW2					
深さ5mm		深さ5mm		深さ5mm		深さ5mm					
破断年月日		破断なし		破断年月日		破断なし					
2002/02/04				2002/02/05							
深さ25mm		深さ25mm		深さ25mm		深さ25mm					
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深さ50mm		深さ50mm		深さ50mm		深さ50mm					
破断なし		破断なし		破断なし		破断なし					
深さ75mm		深さ75mm		深さ75mm		深さ75mm					
破断なし		破断なし		破断なし		破断なし					
腐食センサE3			腐食センサW3			腐食センサE4			腐食センサW4		
深さ5mm			深さ5mm			深さ5mm			深さ5mm		
破断年月日			破断なし			破断なし			破断年月日		
2002/03/06									2002/01/23		
深さ25mm			深さ25mm			深さ25mm			深さ25mm		
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深さ75mm			深さ75mm			深さ75mm			深さ75mm		
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腐食センサ、鉄筋自然電位の詳細データは腐食センサボタンをクリックしてください



Monitoring data of electric resistance on wires having concrete cover thickness ( $x_R$ )

Expected corrosion initiation time on rebar  $\left( \frac{145.5}{5} \right) \cdot 0.55 = 465.7$  年

# Objects of inspection by using monitoring

Type of system	Measurement	Object of Judgment	Threshold value
Embedded type of reference electrode	Natural potential of rebars	Initiation of corrosion	-180 mV vs Ag/AgCl
		Formation of macro cell	50mV of potential difference
Monitoring sensor detecting chloride penetration depth	Depth where chloride content is critical level for initiating rebar corrosion	Chloride penetration depth	75mm
		Alkali Contents in cover concrete	3.0kg/m <sup>3</sup>

# Conclusive Remarks

- **In the construction of the access bridge to Shin-Kitakyusyu Airport, a lot of new trial to keep the durability of structure has been conducted.**
- **In the viewpoint of a preventive maintenance concept, maintenance activity will be performed on the piers from the beginning of the service.**

# Conclusive Remarks

- **The Guideline for the preventive maintenance on the piers has been published. It is the first one in Japan for maintenance of newly constructed structures.**
- **The monitoring system has been installed in the pier for collecting data concerning not only with rebar corrosion but also with chloride penetration depth into concrete, which is detected by using the newest technology developed.**

Thank You



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